

Forrestfield-Airport Link Connect. Fly. Grow.

Forrestfield Station Multi-Storey Car Park Traffic Impact Assessment

FAL-AURECON-TM-RPT-00002

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Traffic Impact Assessment

23 September 2019 Revision: 2 Reference: 507233

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Appendices

Appendix A – Proposed Development Plan Appendix B – SIDRA Results

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1 Introduction and Background

Aurecon has been commissioned by Public Transport Authority (PTA) to prepare a Traffic Impact Assessment (TIA) in accordance with the Western Australia Planning Commission – Transport Impact Assessment Guidelines (Volume 4, Individual Developments) for the multi-storey car park as part of the future Forrestfield Station ("Station").

To accommodate the anticipated park-and-ride demand at the station, it had previously been proposed that the multi-storey car park would include a total of 2,500 parking bays. Following subsequent studies relating to the required parking quantum at the Station, this was reduced to 1,249 parking bays as this was considered sufficient to accommodate the "Park and Ride" demand by 2050 due to the TOD-nature of the area.



1.1 Forrestfield Station

The Station is one of the key rail stations for the Forrestfield-Airport Link (FAL) project that was announced by the WA Government in 2014. The new rail route will open up a new rail corridor to the eastern suburbs and foothills, allowing an approximately 20-minute rail journey to Perth city. The FAL is intended to connect and improve the public transport access for the Bayswater, Belmont, Redcliffe and Forrestfield areas.

The Station is located within the Forrestfield North District Structure Plan (FNDSP) area and a Transport Assessment (Revision: Final, dated 31/08/2015) was previously prepared in support of the Structure Plan. While the Station and proposed multi-storey car park were included as part of the FNDSP, changes have since been proposed to the quantum of parking bays within the car park. The analysis undertaken as part of this TIA therefore primarily focuses on the proposed changes to the car parking quantum and re-assesses the traffic impacts to the adjacent transport network. The FNDSP TIA has been used as a reference for this TIA to ensure consistency of analysis methodology and overall structure planning for the area.

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Figure 1-1: Forrestfield Station location

In addition, a transport modelling study (Forrestfield-Airport Link Transport Modelling, Revision: Final, dated 23/04/2015) was undertaken by Aurecon for the FAL project to assess the overall traffic impact from the Station and proposed car park on the local road network in Forrestfield. As part of this study, the traffic generation from the adjacent land use changes proposed as part of the FNDSP were included in the model. The study area for the FAL transport modelling study is shown in Figure 1-1. The FAL transport modelling report has been used as another reference for this TIA to ensure consistency of future non-development (background) traffic flows.



Figure 1-2: Forrestfield-Airport Link Transport Modelling study area

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2 Existing Situation

2.1 Development Site and Surrounding Land Use

The development site is currently unoccupied and is surrounded by Dundas Road, Dundas Road Link, Maida Vale Road, and Ibis Place. The existing land use surrounding the site (within 400m radius) is predominantly light industrial, with some residential areas and vacant lands as shown in Figure 2-1. Within a 800m radius of the development site, there are additional residential dwellings to the north and north-east, rural residential dwellings to the south-east, and additional industrial areas to the south and west.



Figure 2-1: Existing land uses in the vicinity of the site

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City of Kalamunda



2.2 Planning Context

2.2.1 City of Kalamunda Town Planning Scheme No. 3

As defined by the City of Kalamunda Town Planning Scheme No.3, the proposed development site is designated as industrial development.



Figure 2-2: Planning zone (source: City of Kalamunda Town Planning Scheme No. 3)

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2.2.2 Forrestfield North District Structure Plan

As shown in Figure 2-3, the proposed development site is designated as "Railway" land use in the FNDSP. The proposed car park is intended to only accommodate passengers using the future Forrestfield Station, which is consistent with the intended land use as part of the FNDSP.



Figure 2-3: Proposed land use (Source: Forrestfield North District Structure Plan, 02/09/2015)

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2.3 Existing Road Network

The proposed development site is located adjacent to Dundas Road, with Maida Vale Road, Ibis Place and Sultana Road West bordering the proposed site.

Under the Main Roads functional road hierarchy, as illustrated in Figure 2-4, Dundas Road, Dundas Road Link and Maida Vale Road are classified as Distributor B roads, while Milner Road is classified as a Local Distributor, and both Ibis Place and Sultana Road West are classified as Access Roads.



Figure 2-4: Main Roads WA Functional Road Hierarchy (Source: Main Roads Mapping Information System) The characteristics of the key roads in the vicinity of the proposed development site are summarised below:

Dundas Road

Dundas Road is currently a single carriageway with one lane in each direction between Tonkin Highway and Abernethy Road. Dundas Road carries approximately 4,800 vehicles/day (as recorded by MRWA in 2016/17), although it is noted that the volume of traffic on Dundas Road has likely changes since construction of the Station has commenced. The section of Dundas Road between Main Vale Road and Imperial Street has been removed as part of the proposed development for the Station, with Dundas Road Link being constructed to replace Dundas Road north-south connectivity. Dundas Road has a posted speed limit of 70km/h.

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Dundas Road Link

Dundas Road Link consists of a single undivided carriageway with one lane in each direction, joining Dundas Road to the north and south of the proposed site.

Maida Vale Road

Maida Vale consists of a single undivided carriageway with one lane, and is located between Dundas Road and Hawtin Road. Maida Vale Road has a posted speed limit of 60 km/h and carries approximately 4,200 vehicles/day (west of Milner Road) as recorded by City of Kalamunda in 2013/14.

Milner Road

Milner Road is an undivided single carriageway with one lane in each direction and is located between Berkshire Road and Maida Vale Road. Milner Road has a posted speed limit of 70km/ and carries approximately 2,500 vehicles/day (north of Stewart Road).

Ibis Place

Ibis Place is an undivided single carriageway within one lane in each direction. Ibis Place is currently only connected to Maida Vale Road. Ibis Place has a speed limit of 50km/h. As part of the Station construction, modification are currently being undertaken to Ibis Place, which has therefore been closed for public use.

2.4 Existing Intersections

At a meeting with the City of Kalamunda and the Public Transport Authority, the following existing intersections were identified as potentially impacted by the proposed Forrestfield Station, and have therefore been assessed as part of this study:

- Dundas Road Link/ Dundas Road north Priority controlled three-way intersection with Dundas Road Link controlled by a yield/ give-way sign;
- 2. Dundas Road/ Maida Vale Road/ Construction Site Access Priority controlled three-way intersection with Main Vale Road controlled by a yield/ give way sign;
- 3. Maida Vale Road/ Ibis Place Priority controlled three-way intersection with Ibis Place controlled by yield/ give-way sign;
- 4. Milner Road/ Sultana Road West Priority controlled four-way intersection with Sultana Road West controlled by a yield/ give-way sign.

These intersections are shown in Figure 2-5. In addition to the above intersections, three additional (future) key intersections have also been assessed as part of this study (refer to Section 7.2).

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Figure 2-5: Key intersections within the study area

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2.5 Existing Public Transport

There are currently no public transport services in the immediate vicinity of the proposed development site. As shown in Figure 2-6, the bus stop located closest to the site currently is approximately 600m north of the site and is serviced by bus service 298, which operates between the Elizabeth Quay Bus Station and a bus stop near the Hawtin Road/ Kalamunda Road intersection (east of Roe Highway). This bus service operates at low frequency that only provides three services to and from Perth in the weekday morning and evening peak periods.



Figure 2-6: Public transport services within 800m of the proposed site (source: Transperth)

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2.6 Existing Pedestrian and Cycling Facilities

In the vicinity of the proposed development site, there is currently a 1.5m wide footpath along the eastern side of Dundas Road, joining the northern side of Maida Vale Road. The footpaths along both Dundas Road and Maida Vale Road are still considered adequate for to service the existing pedestrian use.

As shown in Figure 2-7, Maida Vale Road is classified as having "good road riding environment" which suitable for on-street cycling, connecting to the other shared paths located further north.



Figure 2-7: Canning and Armadale: Perth Bike Map (source: DoT, published October 2016)

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3 Proposed Development

The proposed multi-storey car park will include the following:

- 52 motorcycle bays;
- 12 disability parking bays;
- 12 electric parking bays;
- 33 short-term parking bays; and
- 1,140 standard parking bays.

The layouts for the proposed multi-storey car park are included in Appendix A.

3.1 Accessible Parking Requirements

The minimum accessible parking requirements for the site are specified in the National Construction Code 2016. As shown in Table 3-1, a total of 12 accessible parking bays are proposed as part of the development and therefore complies with the minimum accessible parking requirements. A non-parking car bay will also be provided adjacent to all the disability parking bays as recommended in the *AS2890.6 Off-street Parking for People with Disabilities* guideline.

Table 3-1: Accessible Parking Requirement

User Class	Minimum Parking Rates	Car parking Provision	Minimum Accessible Parking Required	Accessible Parking Provision
Car park (Class 7a)	1 bay for every 100 car parking bays	1,185	12	12

3.2 Motorcycle Bays

The parking facility is proposed to include a total of 52 motorcycle parking bays.

3.3 Parking and Access Arrangement

Access to the proposed car park is proposed via a single entry (Access 1) on the proposed new service road (south of Dundas Road/ Maida Vale Road intersection). While vehicles will also be able to exit the parking facility at Access 1, a left-out exit is also proposed on Ibis Place (Access 2) as shown in Figure 3-1. Access 1 will be constructed to only allow left-in left-out (LILO) turn movements.

The proposed car park will include electronic signage at Access 1. This signage will display the number of available bays in order to reduce the amount of vehicles recirculating through the car park and hence reduce any congestion.

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Figure 3-1: Proposed Car Park Access Arrangements

It is understood that the proposed accesses will not include any physical barriers (e.g. boom-gates) in order to minimise any traffic impact on the external road network. While it is noted that PTA may consider introduction of physical barriers at Access 1 in the future, such barriers have not been considered as part of this assessment.

3.4 Kiss and Ride Facility

As shown in Figure 3-1, the proposed service road will also include a Kiss and Ride facility on the western side of the road and can accommodate up to 10 vehicles parked at any one time.

3.5 Pedestrian Movements

The proposed passengers exit/ entry desire walking lines are shown in Figure 3-2. Passengers will be directed to enter/exit the proposed car park via the main pedestrian entrance located at the south-west.

Apart from the vertical transport adjacent to the main entrance, staircases are available at the corners of north-west, north-east and south-east, to access the each of the different levels. Pedestrian crossings are also proposed across the circulation lanes adjacent to vehicle accesses on ground floor.

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Figure 3-2: Pedestrian desired movements

3.6 End-of-trip Facilities

The proposed car park will not include any bicycle parking spaces as these will be provided at the Station in the form of bicycle shelters and U-rails. The Station is not designed to be the final destination for passengers and no shower or change-room facilities are proposed as part of the car park.

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4 Changes to the Surrounding Area

4.1 Forrestfield North District Structure Plan (FNDSP)

The purpose of the structure plan is primarily to guide development around the proposed Forrestfield Station. This structure plan is progressed as two key components; the Transit Oriented Development (TOD) Precinct and Residential Precinct. As shown in Figure 4-1, the proposed development site is located within the TOD Precinct.



Figure 4-1: Forrestfield North District Structure Plan (source: <u>http://www.kalamunda.wa.gov.au/Services/Planning/Major-Projects/Forrestfield-NORTH#dcp</u>)

From the DNSCP structure plan as shown in Figure 4-2, the proposed development site will be surrounded by the Station (railway land use) to the west and mixed-use developments to the south and north, with a park and recreation area located to the west.

As part of the TOD Precinct, new shared paths are proposed along most of the corridors in the vicinity of proposed site, with the exception of the Dundas Road Link.

On-street cycle paths are also proposed along Sultana Road West, Milner Road, and the new corridor to the north of Sultana Road West. The proposed walking and cycling facilities will provide new connectivity to land uses in all direction, encouraging active mode transport as one of the crucial aspects for the TOD design.

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The FNDSP also proposes some upgrades to the road network to improve the traffic circulation and connectivity towards the proposed land uses in and around the TOD, as well as the Station. The proposed intersections upgrades are as shown in Figure 4-3, while the proposed road network changes that will directly impact on the vicinity of proposed site include:

- Extension of Raven Street towards north and intersect at Maida Vale Road as a new prioritycontrolled intersection, with the east-west movement as the main movement;
- Realignment of Ibis Place and Maida Road on the north-east of proposed development, such that Ibis Place will become the main movement at the intersection;
- In conjunction with proposed development, removal of the northern section of Dundas Road, with the new service road to be constructed instead to provide access to the proposed development and Kiss and Ride facility, with a roundabout to be constructed at the Dundas Road/ Maida Vale Road intersection (see Figure 4-4);
- Construction of a new connection between Ibis Place and Sultana Road West
- The staggered Milner Road/ Sultana Road West intersection to be upgraded to a four-way signalised intersection (refer to Figure 4-3); and
- Sultana Road West extension to connect Sultana Road East with a fly-over Roe Highway Bridge (refer to Figure 4-2).

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Figure 4-2: Forrestfield North District Structure Plan (City of Kalamunda)

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Figure 4-3: Proposed Intersection Plan within FNDSP

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Figure 4-4: Kiss and ride facilities (sourced: PTA)

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4.2 Forrestfield Station

Forrestfield Station is one of the key stations for the FAL, with the proposed car park to accommodate the "Park and Ride" demand. The Station has been designed to provide more public transport options for the residents within City of Kalamunda. It is expected the station demand will be largely driven by the residents as the surrounding land uses are generally residential. The industrial land uses in the vicinity are unlikely to generate substantial passenger numbers alighting at the Station. The Station is estimated to be operational prior to 2021, along with the other two stations (Airport Central Station and Redcliffe Station). The Station layout is shown in Figure 4-5.



Figure 4-5: Proposed Forrestfield Station layout (source: PTA)

4.2.1 Proposed Land Use

The Forrestfield Station will consist an at-grade rail station, bus station with eight active and four layover bus bays, and bicycle parking spaces.

4.2.2 Additional Potential Parking Facility

As the urban land use within Kalamunda and Mundaring develops and demand for the parking facilities increase, an allowance has been made as part of the Station design for an additional at-grade parking facility to the south of the proposed development site. This additional parking facility is intended to include up to 600 additional parking bays but will only be developed in the long-term if/when the capacity of the parking facility assessed as part of the TIA is exceeded.

4.2.3 Walking and Cycling Access

As shown in Figure 4-6, the pedestrian and cyclist access will be via a network of shared paths that will integrate with the proposed FNDSP.

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Figure 4-6: Walking and Cycling Access to Forrestfield Station (source: PTA)

4.2.4 Public Transport Access

Bus passengers will arrive at the bus station adjacent to the train station. The location of the active stands are designed to prevent alighting passengers to cross any roads in order to access the rail station. The buses will access the station from Ibis Place as shown in Figure 4-7.



Figure 4-7: Public Transport Access to Forrestfield Station (source: PTA)

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5 Integration with Surrounding Area

5.1 Surrounding Major Attractors and Generators

There are currently few major attractors and generators surrounding the area as the freight yards to the west create a barrier to movement in this direction.

5.2 **Proposed Changes to Land Uses within 1,200m**

There will be major land use changes to the east and south-east of the proposed site. The areas covered by FNDSP are estimated to result in the development yields summarised in Table 5-1.

Development Type	Total Area (m ²)	No. of Dwellings
Mixed Use	197,000	2,400
Industrial	672,000	-
Residential	534,000	2,800
Retail	107,000	-

Table 5-1: Estimated	Land	Use	Changes	within	1,200m
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5.3 Adequacy of Existing Transport Networks

5.3.1 Pedestrian and Cycle Network

The existing pedestrian and cycle network is considered sufficient considering the existing population and employment densities within the area.

5.3.2 Public Transport

The existing public transport network of bus services is not considered adequate for the area as most of the area is currently not served by any public transport services. This is however will be improved significantly with the future high bus frequency at the Station.

5.3.3 Road Network

The existing road network is considered adequate to accommodate the current traffic demand.

5.4 Deficiency in Existing Transport Networks

5.4.1 Pedestrian and Cycling Network

With the Station anticipated to open prior to 2021, it is expected to attract significant pedestrians and cyclists along Ibis Place, Maida Vale Road, Imperial Street and Sultana Road West. These trips are likely to be generated from the adjacent residential areas in High Wycombe. The existing pedestrian network and cyclist network are considered inadequate for the anticipated pedestrian and cyclist trips.

5.4.2 Road Network

Sultana Road West section to the west of Milner Road is currently not adequate for the expected traffic volume generated for the proposed car park and Station. The road currently has a cul-de-sac and will not provide circulation for traffic within the area.

Ibis Place is also not designed to accommodate the expected future traffic growth and the current culde-sac would require removal in order to provide the circulation necessary within the area.

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6 Road Safety

The recorded crash data from January 2014 to December 2018, sourced from Main Roads, was analysed to determine any significant trends within the area immediately surrounding the proposed development site. The corridors including:

- Dundas Road (between Sorensen Road and Berkshire Road);
- Maida Vale Road (between Dunas Road and Milner Road);
- Milner Road;
- Sultana Road West;
- Raven Street; and
- Ibis Place.

A summary of the crash data is presented in Table 6-1.

Table 6-1: Summary of Crash Statistics for Adjacent Roads

Crash Nature		Total			
	PDO Minor	PDO Major	Medical	Hospital	
Hit Object	2	2			4
Non-Collison	1				1
Rear End	4	14			18
Right Angle		12	2	4	18
Sideswipe Same Direction		2			2
Total	7	30	2	4	43

* Property Damage Only (PDO)

As shown in Table 6-1, more than 80% of the crashes have resulted in Property Damage Only (PDO), and the majority of crashes are classified as rear-end and right-angle crashes. It is noted that there were two crashes that required medical attention and four crashes that resulted in hospitalisation, with all of these crashes occurring at the Dundas Road/ Berkshire Road intersection and midblock of Milner Road (north of Berkshire Road). These crashes are likely due to the relatively short distance between Dundas Road/ Berkshire Road and Berkshire Road/ Milner Road intersections that are currently priority-controlled. As the intersections are proposed to be signalised in the future (refer to Figure 4-3), this is anticipated to improve the driver awareness and reduce the crash rates.

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7 Traffic Analysis

7.1 Forrestfield- Airport Link Transport Modelling

PTA had previously commissioned Aurecon to undertake a transport modelling study for the Forrestfield-Airport Link. The model was developed in an Aimsun hybrid model, containing both microscopic and mesoscopic model area as shown in Figure 7-1.



Figure 7-1: Forrestfield-Airport Link Transport Modelling study area

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The model consists of two peak periods; AM Peak (07:30am-08:30am) and PM Peak (16:30-17:30pm). It was calibrated to the 2014 observed traffic behaviour and forecast models developed for the following future scenarios with associated infrastructure changes.

- 2017 Forrestfield-Airport Link Construction Year
 - Gateway WA grade separations
 - Closure of Dundas Road, and replace with Dundas Road Link to the west
- 2021 Forrestfield-Airport Station Opening Year
 - Completion of FAL;
 - Completion of Forrestfield Station and proposed car park (previously assumed to include 2,500 parking bays)
 - Diversion of Maida Vale Road towards the Station
 - Road network changes in the FNDSP
- 2031 MRWA Future Year
 - Road network modifications for FNDSP
 - North facing ramps at Roe Highway/ Maida Vale Road
 - Grade separation of Roe Highway/ Kalamunda Road Interchange
 - Minor layout changes at Dundas Road/ Daddow Road intersection

The future year models incorporated the FNDSP proposed developments and, while the MRWA strategic model (ROM24) was used to capture the traffic associated with developments in the wider area, such as the DA6 plans for Redcliffe and Perth Airport Master Plan as indicated in Table 7-1 and Table 7-2 (including FNDSP).

Table 7-1: 2021 Land Use in Addition to 2021 ROM24 for FAL

Area	ROM	Population	Total Private		m²	
Alea	Zone	Dwellings	Dwellings	Manufacturing	Retail	Commercial
Kalamunda	585	As is	As is	As is	As is	As is
Airport N	597	As is	As is	As is	As is	1,050,000
Airport W	598	As is	As is	As is	As is	As is
Airport S	600	As is	As is	As is	As is	1,150,000
CoB	603	As is	As is	As is	As is	As is

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Aroa	ROM	Population	Total Brivato		m²	
Alea	Zone	Dwellings	Dwellings	Manufacturing	Retail	Commercial
Kalamunda*	585	10,388	5,743	134,416	As is	413,117
Airport N	597	As is	As is	120,000	As is	1,290,000
Airport W	598	180	100	50,000	As is	169,000
Airport S	600	720	400	100,000	As is	1,150,000
CoB**	603	1,561	867	As is	3,000	4,000

Table 7-2: 2031 Land Use in Addition to 2021 ROM24 for FAL

* Included PTA long-term 600 parking bays; ** City of Belmont; *** Included FNDSP

The model has captured the essential future developments and road infrastructure within the vicinity of proposed site, as well as in the wider network. Therefore, this model has been used as the main reference for this TIA to determine future background traffic to ensure traffic generation from these developments are captured in the intersection analysis.

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7.2 Key Intersections

The following seven intersections (see Figure 7-2) within the proximity of the proposed site have been assessed as part of this analysis:

- 1. Dundas Road north/ Dundas Road Link
- 2. Dundas Road/ Maida Vale Road
- 3. Maida Vale Road/ Ibis Place
- 4. Milner Road/ Sultana Road West
- 5. Maida Vale Road/ Raven Street
- 6. Proposed Car park Access 1
- 7. Proposed Car park Access 2



Figure 7-2: Key Intersections within Study Area

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7.3 Assessment Years

The analysis has considered the following assessment years:

- Existing scenario represents the current situation without any development traffic (i.e. current geometry layouts as captured in the FAL model)
- Year of opening 2021, based on the assumption that the car park will be opened at the same time as the Station (prior to 2021).
- The future year 2031, to assess the traffic impact of 10-year forecast horizon after the year of opening.

7.4 Time Periods

To ensure the peak periods for the proposed car park are reasonably representative of a typical park and ride train station, two existing train station car parks demand profiles (Warwick Station and Stirling Station), were sourced.

The car park at the Warwick Station is assumed to operate in a similar manner to the proposed Forrestfield Station car park as it has approximately 1,200 bays and is located at a similar distance away from the Perth CBD, and the time taken to travel between the station and the Perth station is also assumed to be similar. The Stirling Station carpark (also ~1,200 bays) is slightly closer to the city but provides an additional reference.

As both of these car parks are accessed from signalised intersections, SCATS traffic data was sourced from these signalised intersections in order to compare against the existing daily traffic volume on Dundas Road. The analysis undertaken indicated that the peak parking profile coincides with the Dundas Road peak in the morning (7-8AM), while the evening peak (5-6PM) occurs slightly later than the Dundas Road peak (3.30-4.30PM), as shown in Figure 7-3.



Figure 7-3: Traffic Profiles for Stirling Station and Warwick Station

As previously described, a microsimulation model (Aimsun) was developed as part of the Forrestfield-Airport project. The microsimulation model was developed for two peak periods, AM Peak (7.30-8.30AM) and PM Peak (4.30-5.30PM). While it was observed that the existing car park peak demands occur slightly earlier and later than the model peak periods (by 30mins), it is assumed the two peaks coincide to provide a worst-case demand scenario. While it is noted that the Dundas Road PM peak occurs slightly earlier, this is representative of existing demand, and it is considered likely the future

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demand will shift towards wider network peak (4.30-5.30PM) once traffic generation from the proposed car parks and surrounding developments are accounted for.

Therefore, the following peak hours have been assessed as part of this analysis:

- AM Peak: 07:30AM 08:30am
- PM Peak: 4:30PM 5:30PM

7.5 Scenarios

As agreed with the City of Kalamunda and PTA, Aurecon has undertaken the analysis for the four scenarios shown in Table 7-3.

Table 7-3: Scenarios for traffic analysis

Scenario	Details and assumptions	Intersections Assessed (refer Figure 7-2)	
Scenario 1 – Existing Conditions	Traffic volumes from the FAL model	1, 2, 3 and 4	
Scenario 2A – 2021 (Year of Opening) without Raven Street	 2021 background (non- development) traffic volumes from the FAL model 	1, 2, 3, 4, 6 and 7	
extension	 Proposed development in place (1,249 parking bays), with Forrestfield Station open 		
	 Without Raven Street extension 		
Scenario 2B – 2021(Year of Opening) with Raven Street	 2021 background (non- development) traffic volumes from the FAL model 	1, 2, 3, 4, 5, 6 and 7	
extension	 Proposed development in place (1,249 parking bays), with Forrestfield Station open 		
	 With Raven Street extension to Maida Vale Road 		
Scenario 3 – 2031: 10 years after opening	 2021 background (non- development) traffic volumes from the FAL model 	1, 2, 3, 4, 5, 6 and 7	
	 Proposed development in place (1,249 parking bays), with Forrestfield Station open 		
	 Additional 600-bay parking facility in place 		

7.6 Background Traffic

As previously described, the FAL Aimsun model has been used as one of the key sources of data relating to traffic volumes for the purpose of this assessment. This model has been adopted to ensure the that the traffic associated with the future developments within the surrounding areas are accounted for, as well as any wider transport network impacts. It is noted that the traffic associated with the parking facility (previously assumed to include 2,500 parking bays) was removed from the model.

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7.6.1 Scenario 1 – Existing Conditions

The FAL Aimsun model was calibrated to 2014 observed traffic volumes and forecasted to year 2017. The 2017 model included the committed changes between 2014 and 2017 as described in Section 7.1. This TIA has used the intersection turn volumes extracted from the model shown in Figure 7-4. Note that only two of the key intersections were accessed in this scenario as the remaining intersections are either cul-de-sac or temporarily closed for public use.

7.6.2 Scenario 2 – 2021: Year of Opening

Intersection turn volumes from the 2021 FAL Aimsun model, summarised in Figure 7-5, were used for the background (non-development) traffic assumptions for Scenario 2A (i.e. no Raven Street extension).

A copy of the 2021 FAL Aimsun model was modified to include the Raven Street extension, with the intersection turn volumes extracted from this model used for the background traffic for Scenario 2B. The intersection turn volumes for this scenario are summarised in Figure 7-6.

7.6.3 Scenario 3 – 2031: 10 Years after Opening

The Scenario 3 background traffic volumes extracted from the 2031 FAL Aimsun Model are summarised in Figure 7-7. It is noted that this model also included the additional 600 parking bays proposed by PTA for the Station located adjacent to Sultana Road West. The trip generation assumed for this additional parking facility is the same as the assumed trip generation for the car park assessed in this TIA.

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Figure 7-5: Adopted Scenario 2A (2021) background turning volume

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Figure 7-6: Adopted Scenario 2B (2021) background turning volume

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West/ Milner Road					
es	t (NW)			
	7	0			
	4	0		1 (1)(5)	
	7	4	Milner F	Road (NE)	
	, 193	99			
	10	9			
es	t (SE)				
ba	d/ Ra	ven Str	eet		
			Maida V	ale Road	
T	r	69	38		
L		43	40		



Figure 7-7: Adopted Scenario 3 (2031) background turning volume

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7.7 Proposed Development Trip Generation, Distribution and Assignment

7.7.1 Trip Generation

PTA estimated the number of one-way boardings for the Forrestfield Station, with the resulting traffic generation from these trips summarised in Table 7-4. The estimated traffic generation for the parking facility for the peak hours (as a ratio of the number of vehicles going to/from the parking facility compared to the total number of parking bays available) was assumed to be approximately 70%. This is close to the observed ratio from the Warwick Station and Stirling Station, which was found to be approximately 75% and 65% for the AM and PM peak hours respectively.

Purposo	Estimated One	AM Peak		PM Peak	
ruipose	(car trips)	Inbound	Outbound	Inbound	Outbound
Park and Ride	1,200	840	-	-	840
Kiss and Ride	750	500	500	500	500
Total	1,950	1,340	500	500	1,340

Table 7-4: Assumed Trip Generation for the Proposed Car Park

7.7.2 Proposed Development Traffic Distribution

Park and Ride

Based on the existing and future developments in the area, the following assumptions were adopted with regards to the traffic distribution for the proposed parking facility:

- Both AM and PM peak periods have similar traffic distribution;
- All trips are assumed to return to their origins/ destinations;
- The proposed car park is expected to attract the majority of the trips from the wider network rather in its vicinity;
- Trips attracted and generated from the north residential areas are assumed to be 30% and likely to reach the site via either Dundas Road (10%) or Maida Vale Road (20%);
- Trips attracted and generated from the residential areas within the FNDSP are assumed to be 15% and assumed to reach the site via either Raven Street (5%) or Sultana Road West (10%);
- Trips attracted and generated from the further east and northeast are assumed to be 25% and assumed to reach the site via Maida Vale Road; and
- Trips attracted and generated from the residential areas to the south and southeast are assumed to be 30% and assumed to reach the site via Dundas Road south (20%) and Sultana Road West (10%).

All trips going to proposed site (Access 1) are assumed use the most direct routes, while 70% of the outbound trips were assumed to exit via Access 1, with the remaining 30% assumed to exit via Access 2.

The above traffic distribution assumptions are displayed visually in Table 7-5 while the resulting development traffic volumes at the key intersection summarised in Figure 7-8.

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Table 7-5: Adopted overall park and ride inbound/ outbound site traffic distribution

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Figure 7-8: Summary of Development Traffic

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ad/ Raven Street
Maida Vale Road
ad/ Proposed Access 1
Propsoed Car Park Access 1
posed Access 2
Ibis Place
0 198



7.7.3 Kiss and Ride

As shown in Figure 4-4, the Station (Area 2) will also include an additional Kiss and Ride facility adjacent to the bus stop area.

It is assumed that the traffic distribution for Kiss and Ride facilities will distribute towards the facilities as shown in Table 7-6, resulting in the intersection turn volumes shown in Table 7-7 and Figure 7-9 respectively. The majority of the Kiss and Ride traffic is expected to utilise the facility adjacent to the station rather the proposed site (Area 1), given Dundas Road/ Maida Vale Road intersection is expected to carry high traffic demand.

Table 7-6: Adopted splits	between the facility at	proposed site and the Station
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From/To	Proposed Site (Area 1)	Forrestfield Station (Area 2)	Total
Dundas Road South	40%	60%	100%
Dundas Road North	100%	0%	100%
Maida Vale Road	40%	60%	100%
Sultana Road West	0%	100%	100%
Raven Street	0%	100%	100%

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Table 7-7: Adopted overall kiss and ride inbound/ outbound site traffic distribution

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Figure 7-9: Intersection Turn Volumes from Kiss and Ride Facilities

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	Raven	Street	
		Maida Vale	Road
T	90	90	
L	0	0	
ad/	Propos	ed Access	1
ad			
	Prop	soed Car	
0	Park	Access 1	
pos	ed Aco	cess 2	
ed Ca	ar Park A	ccess 2	
		Ibis Pla	ce
	0	Ibis Pla	<u>c</u> e
	0	Ibis Pla	<u>ce</u>
	0	Ibis Pla 0	<u>c</u> e
	0	Ibis Pla	<u>ce</u>
	0	Ibis Pla 0	<u>c</u> e
	0	Ibis Pla 0	<u>c</u> e
	0	Ibis Pla 0	ce
	0	Ibis Pla 0	ce
	0	Ibis Pla 0	
	0	Ibis Pla 0	ce
	0	Ibis Pla 0	

7.7.4 Overall Traffic Demand (with Raven Street Extension)

The overall traffic demand that to/from the proposed car park at the key intersections are summarised in Figure 7-10 for the scenarios that include the Raven Street extension.



Figure 7-10: Summary of Development Generated Traffic (with Raven Street Extension)

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ad/ Raven Street				
		Maida V	ale Road	
Г	468	90		
L	0	0		
ad/	Propos	sed Acc	ess 1	
P	ropsoed	Car ss 1		
Ľ	un Acce	331		
pos	sed Ac	cess 2		
Car	Park Acc	ess 2		
		Ibis Pla	ace	
0	168	3		



Without Raven Street

The overall traffic demand that to/from the proposed car park at the key intersections are summarised in Figure 7-11 for the scenarios that don't include the Raven Street extension.



Figure 7-11: Summary of Development Generated Traffic (without Raven Street Extension)

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ad/ Raven Street
Maida Vale Road
520 100
0 0
ad/ Proposed Access 1
d
Propsoed Car
Park Access 1
pposed Access 2
d Car Park Access 2
Ibis Place
0 168



7.8 **Total Intersection Turn Volumes**

Scenario 1 – Existing Condition 7.8.1

As described in Section 7.6.1, the intersection turn volume for Scenario 1 are summarised in Figure 7-4.

7.8.2 Scenario 2A – Year of Opening 2021 without Raven Street Extension

For Scenario 2A, it is assumed that the proposed multi-storey car park will have been completed, along with the completion of Forrestfield Station. In this scenario, the Raven Street extension is assumed not to have completed. The combined intersection turn volumes for this scenario are summarised in Figure 7-12.



Figure 7-12: Summary of Intersection Turn Volumes for Scenario 2A

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Maida Vale Road
651 188 0 0
oad/ Proposed Access 1
Propsoed Car
Park Access 1
oposed Access 2
l Car Park Access 2
Ibic Place
87 250

7.8.3 Scenario 2B – Year of Opening 2021 with Raven Street Extension

For Scenario 2B, it is assumed that the proposed multi-storey car park will be completed, along with the completion of Forrestfield Station. In this scenario, the Raven Street extension is assumed to have completed. The combined intersection turn volumes for this scenario are summarised in Figure 7-13.



Figure 7-13: Summary of Intersection Turn Volumes for Scenario 2B

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ad/ F	Raven	Street
		Maida Vale Road
T	537	128
L	43	40
ad/ I		
pad/ r	ropos	ed Access 1
Prop	soed Car	
Park	Access 1	<u> </u>
opose	ed Acc	ess 2
Car Pa	k Access	2
48	210	Ibis Place
40	210	I

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7.8.4 Scenario 3 – 10 Years after Opening (2031)

The combined intersection turn volumes for Scenario 3 are summarised in Figure 7-14.



Figure 7-14: Summary of Intersection Turn Volumes for Scenario 3

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ad/ I	Rave	n Street
т	571	Maida Vale Road
L	33	37
ad/ I	Prop	osed Access 1
Pro Par	psoed k Acces	Car ss 1
opos	ed A	ccess 2
l Car P	ark Ao	cess 2
147	19	Ibis Place

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7.9 SIDRA Assessment

SIDRA analysis has been undertaken for the key intersections near the site, (see Figure 7-2) including:

- 1. Dundas Road north/ Dundas Road Link
- 5. Maida Vale Road/ Raven Street
- 6. Proposed Car park Access 1
- 7. Proposed Car park Access 2
- 4. Milner Road/ Sultana Road West

2. Dunas Road/ Maida Vale Road

3. Maida Vale Road/ Ibis Place

Intersection capacity analysis was undertaken in SIDRA (version 8), to evaluate the intersection performance for the various scenarios. The SIDRA vehicle and model parameters were set up in accordance with the suggested parameters in the Main Roads WA Operational Modelling Guidelines for Light and Heavy vehicles, with buses considered as part of the Heavy vehicles.

Based on the data collected at the existing train stations; Warwick Station and Stirling Station, it was found that the Peak Flow Factor (PFF) for the car parks are approximately 89% and 91% (based on 15-minute intervals). This TIA has adopted more conservative approach of adopting the 89% peak flow factor for both the AM and PM peak periods.

The layout configuration for all intersections are shown in Figure 7-15 to Figure 7-21.



Figure 7-15: Intersection 1 – Dundas Road north/ Dundas Road Link for all Scenarios

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Figure 7-18: Intersection 4 – Milner Road/ Sultana Road West for Scenario 1, 2A, 2B (left) and 3 (right)

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Figure 7-19: Intersection 5 - Maida Vale Road/ Raven Street for Scenario 2B and 3





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7.9.1 Scenario 1 – Existing Conditions

Summaries of the intersection performances for Scenario 1 are shown in Figure 7-22 and Figure 7-23 for the AM and PM peak hours respectively, and show that the two key existing intersections are currently operating satisfactory with level of service (LOS) A and low degree of saturation (DOS).



Figure 7-22: Scenario 1 – AM Peak Summary

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Figure 7-23: Scenario 1 – PM Peak Summary

Detailed SIDRA outputs are included in Appendix B.

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7.9.2 Scenario 2A – Year of Opening 2021 without Raven Street Extension

Summaries of the intersection performances for Scenario 2A are shown in Figure 7-24 and Figure 7-25 for the 2021 AM and PM peak hours respectively and indicate that the key intersections will perform satisfactorily with LOS A and low DOS.



Figure 7-24: Scenario 2A – AM Peak Results Summary

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Figure 7-25: Scenario 2A – PM Peak Results Summary

Detailed SIDRA outputs are included in Appendix B.

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7.9.3 Scenario 2B – Year of Opening 2021 with Raven Street Extension

Summaries of the intersection performances for Scenario 2B are shown in Figure 7-26 and Figure 7-27 for the 2021 AM and PM peak hours respectively and indicate that the key intersections will perform satisfactorily with LOS A and low DOS. Overall it is considered that there is only minor differences in the intersection performance in Scenarios 2A and 2B.



Figure 7-26: Scenario 2B – AM Peak Results Summary

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Figure 7-27: Scenario 2B – PM Peak Results Summary

Detailed SIDRA outputs are included in Appendix B.

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7.9.4 Scenario 3 – 10 Years after Opening (2031)

Summaries of the intersection performances for Scenario 3 are shown in Figure 7-28 and Figure 7-29 for Scenario 3 AM and PM peak hours respectively and indicate that the intersections will perform satisfactorily, with the exception of the Milner Road/ Sultana Road West intersection. This intersection is estimated to operate with LOS E and DOS 0.934 for the 2031 AM peak hour, and LOS E and DOS 0.978 for the 2031 PM peak hour. However, it is noted that this assessment conservatively assumes that all pedestrian crossing phases will run in every cycle and therefore underestimates the performance of the intersection.



Figure 7-28: Scenario 3 - AM Peak Results Summary

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Figure 7-29: Scenario 3 – PM Peak Results Summary

The details of the SIDRA outputs are included in Appendix B.

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7.10 SIDRA Results Summary

The intersection performances for the various scenarios are summarised in Table 7-8 and Table 7-9 for the AM and PM peaks respectively.

Table 7-8: Summary of Intersection Performances – AM Peak

Intersection	Output	Scenario 1	Scenario 2A	Scenario 2B	Scenario 3
Dundos Dal north / Dundos Dal Link	LOS	A	A	A	A
Dundas Ro nonn/ Dundas Ro Link	DOS	0.101	0.208	0.211	0.370
Dundon Dd/ Moide Vala Dd	LOS	-	A	A	A
Dundas Ro/ Maida Vale Ro	DOS	-	0.772	0.763	0.826
Maida Vala Dd/ Ibia Dl	LOS	-	A	A	A
Maida Vale Rd/ Ibis Pi	DOS	-	0.406	0.408	0.455
Milner Road (Sultana Road West	LOS	A	A	A	E
Williner Road/ Sultana Road West	DOS	0.184	0.313	0.333	0.934
Maida Vala Dd/ Davan St	LOS	-	-	A	A
Maida vale Rd/ Raven St	DOS	-	-	0.379	0.403
Dranaged Car Dark Assess 1	LOS	-	A	A	A
Proposed Car Park Access 1	DOS	-	0.484	0.484	0.484
Dropood Cor Dark Access 2	LOS	-	A	A	A
FIDPOSED CALFAIR ACCESS 2	DOS	-	0.301	0.307	0.352

Table 7-9: Summary of Intersection Performances – PM Peak

Intersection	Output	Scenario 1	Scenario 2A	Scenario 2B	Scenario 3
Dundes Dal north / Dundes Dal Link	LOS	A	A	A	A
Dundas Ro north/ Dundas Ro Link	DOS	0.176	0.371	0.358	0.335
Dundan Dd/ Maida Vala Dd	LOS	-	A	A	A
Dundas Ru/ Maida Vale Ru	DOS	-	0.641	0.603	0.752
Maida Vala Dd/ Ibia Dl	LOS	-	A	A	A
Maida Vale Rd/ Ibis Pi	DOS	-	0.475	0.472	0.443
Milner Dood/ Sultana Dood West	LOS	A	A	A	E
Milliner Road/ Sultana Road West	DOS	0.142	0.512	0.469	0.978
Maida Vala Dd/ Davian St	LOS	-	-	A	A
Maida vale Rd/ Raven St	DOS	-	-	0.497	0.544
Duran and One Dark Assess 4	LOS	-	A	A	A
Proposed Car Park Access 1	DOS	-	0.491	0.491	0.491
Dranged Car Dark Assess 2	LOS	-	A	A	A
Proposed Car Park Access 2	DOS	-	0.239	0.234	0.289

As shown in the above tables, all intersections are estimated to operate satisfactorily all scenarios, with the exception of the Milner Road/ Sultana Road West intersection. However, as previously noted, this assessment conservatively assumes that all pedestrian crossing phases will run in every cycle (see Figure 7-30and Figure 7-31) and therefore likely overestimates the delays at this intersection.

The details of the SIDRA outputs are included in Appendix B.

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Figure 7-30: Assumed signal phasing sequence for Milner Road/ Sultana Road West for Scenario 3 (2031) AM Peak



Figure 7-31: Assumed signal phasing sequence for Milner Road/ Sultana Road West for Scenario 3 (2031) PM Peak

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Forrestfield Station Multi-Storey Car Park

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8 Summary

The following conclusions have been reached as part of the analysis and assessment undertaken for the proposed multi-storey car park proposed as part of the Forrestfield Station:

- Public Transport: The existing public transport coverage in the area is considered poor as there are only limited services available within 800m walking distance. This will be substantially improved when the Forrestfield Station opens, as this will provide a high-quality rail connection, as well as a substantial increase in the number and frequencies of buses servicing the adjacent areas.
- Pedestrian and Cycling: The existing pedestrian and cycling network is considered adequate to accommodate the existing low-density population and employment densities in the immediate area. As part of the FNDSP, substantial upgrades of the pedestrian and cycling infrastructure is proposed in order to achieve the intended development of the area as a TOD.
- Road Safety: A total of six crashes that have resulted in medical attention (2 crashes) or hospitalisation (4 crashes) were recorded to have occurred in the past 5 years at either the Dundas Road/ Berkshire Road intersection, or midblock on Milner Road close to intersection at Berkshire Road. These crashes are likely to due to the intersections located relatively close. This is expected to be improved with the proposed signalisation of the intersection as part of the FNDSP.
- Road Network: Traffic analysis was undertaken for seven key intersections as part of this assessment. This analysis was undertaken for four scenarios; existing conditions, year of opening (2021) with and without the Raven Street extension, and 2031.
 - Dundas Road north/ Dundas Road Link: The model results indicate that this intersection will
 perform satisfactorily during both the AM and PM peak hours for all scenarios.
 - Dundas Road/ Maida Vale Road: The model results indicate that this intersection will perform satisfactorily during both the AM and PM peak hours for all scenarios.
 - Maida Vale Road/ Ibis Place: The model results indicate that this intersection will perform satisfactorily during both the AM and PM peak hours for all scenarios.
 - Milner Road/ Sultana Road West: While the model results indicate that this intersection will
 operate with LOS E for both the 2031 AM and PM peak hours, this assessment conservatively
 assumes that all pedestrian crossing phases will run in every cycle and therefore likely
 overestimates the delays at this intersection.
 - Maida Vale Road/ Raven Street: The model results indicate that this intersection will perform satisfactorily during both the AM and PM peak hours for all scenarios.
 - Proposed Car park Access 1: The model results indicate that this intersection will perform satisfactorily during both the AM and PM peak hours for all scenarios. However, it is noted that this assessment assumes that the entry lanes for the car park will not include any physical barriers to reduce the risk of vehicles queueing back to the service road. If the installation of physical barriers (such as boomgates) are to be considered at the entry lanes, it is recommended that any potential impacts of these barriers are investigated further to reduce the risk of queues spilling back to the service road.
 - Proposed Car park Access 2: The model results indicate that this intersection will perform satisfactorily during both the AM and PM peak hours for all scenarios.
- Parking Demand: If the parking demand exceeds the 1,249 parking bays available, PTA have made an allowance for an additional parking structure (adjacent to Sultana Road West) that can accommodate up to 600 additional parking bays if required. It is noted that these additional parking bays are not considered to be required until post-2031.

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Appendix A: Proposed Development Plan

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Forrestfield Station Multi-Storey Car Park

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City of Kalamunda

FORRESTFIELD STATION MULTI STOREY CARPARK



Sheet Name

GROUND FLOOR PLAN COVERPAGE LEVEL 1 FLOOR PLAN LEVEL 2 FLOOR PLAN SECTIONS NORTH AND SOUTH ELEVATIONS EAST AND WEST ELEVATIONS SITE PLAN

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Forrestfield Station Multi-Storey Car Park

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	 NOTES REFER TO GHD DRAWINGS FOR THE FOLLOWING ROUND ABOUT, ROAD ENTRY, KISS AND F SETOUT LANDSCAPE (HARD & SOFT) & IRRIGATION BUILDING FENCING & FENCE WALLS STORM WATER BASIN SETOUT & DESIGN EXTERNAL LIGHTING POLES TO PERIMETION NEW TRANSFORMER COMPOUND 	RIDE, DESIGN & N TO ALL SIDES OF ER OF BUILDING
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Forrestfield Station Multi-Storey Car Park

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NOTES



Forrestfield Station Multi-Storey Car Park

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Forrestfield Station Multi-Storey Car Park

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PRECAST CONCRETE WALL WITH TEXTURE PAINT FINISH	
PRECAST CONCRETE WALL WITH SELECT PATTERN TEXTURE PAINT FINISH	
PRECAST COLUMNS - PAINTED	
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Appendix B: SIDRA Results

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Forrestfield Station Multi-Storey Car Park

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City of Kalamunda

SITE LAYOUT

abla Site: 101 [Dundas Road North/ Dundas Road Link_2018_AM]

Dundas Road/ Dundas Road Link_2018_AM Site Category: (None) Giveway / Yield (Two-Way)



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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 76 of 128 Page 76 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2018_AM]

Dundas Road/ Dundas Road Link_2018_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Dundas F	Rd (S)											
1	L2	11	0.0	0.081	5.5	LOS A	0.0	0.0	0.00	0.04	0.00	56.9	
2	T1	145	0.8	0.081	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.4	
Approa	ach	156	0.7	0.081	0.4	NA	0.0	0.0	0.00	0.04	0.00	59.2	
North:	Dundas R	ld (N)											
8	T1	69	0.0	0.101	0.3	LOS A	0.5	4.0	0.27	0.37	0.27	53.5	
9	R2	135	1.7	0.101	5.9	LOS A	0.5	4.0	0.27	0.37	0.27	53.9	
Approa	ach	203	1.1	0.101	4.0	NA	0.5	4.0	0.27	0.37	0.27	53.8	
West: I	Dundas R	d Link (W)											
10	L2	94	4.8	0.067	6.1	LOS A	0.3	2.1	0.24	0.55	0.24	52.5	
12	R2	85	93.4	0.079	7.6	LOS A	0.2	3.0	0.28	0.64	0.28	40.7	
Approa	ach	180	46.9	0.079	6.8	LOS A	0.3	3.0	0.26	0.60	0.26	47.4	
All Veh	icles	539	16.3	0.101	3.9	NA	0.5	4.0	0.19	0.35	0.19	52.5	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 77 of 128 Page 77 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2018_PM]

Dundas Road/ Dundas Road Link_2018_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Dundas F	Rd (S)										
1	L2	10	0.0	0.059	5.5	LOS A	0.0	0.0	0.00	0.05	0.00	56.8
2	T1	104	0.0	0.059	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	59.3
Approa	ach	115	0.0	0.059	0.5	NA	0.0	0.0	0.00	0.05	0.00	59.0
North:	Dundas F	Rd (N)										
8	T1	131	0.0	0.176	0.3	LOS A	1.0	7.2	0.24	0.35	0.24	53.9
9	R2	231	0.5	0.176	5.8	LOS A	1.0	7.2	0.24	0.35	0.24	54.2
Approa	ach	363	0.3	0.176	3.8	NA	1.0	7.2	0.24	0.35	0.24	54.1
West: I	Dundas R	d Link (W)										
10	L2	213	2.6	0.144	5.9	LOS A	0.6	4.8	0.21	0.55	0.21	52.7
12	R2	29	80.8	0.028	7.6	LOS A	0.1	1.0	0.32	0.65	0.32	41.5
Approa	ach	243	12.0	0.144	6.1	LOS A	0.6	4.8	0.23	0.56	0.23	51.6
All Veh	icles	720	4.2	0.176	4.1	NA	1.0	7.2	0.20	0.38	0.20	53.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 78 of 128 Page 78 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2021_AM_noRavenSt]

Dundas Road/ Dundas Road Link_2021_AM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Dundas R	d (S)										
1	L2	48	0.0	0.118	5.5	LOS A	0.0	0.0	0.00	0.14	0.00	55.6
2	T1	162	10.4	0.118	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	57.9
Approa	ich	210	8.0	0.118	1.3	NA	0.0	0.0	0.00	0.14	0.00	57.4
North:	Dundas Ro	d (N)										
8	T1	191	2.9	0.208	0.5	LOS A	1.1	8.4	0.33	0.29	0.33	54.3
9	R2	197	7.4	0.208	6.3	LOS A	1.1	8.4	0.33	0.29	0.33	53.9
Approa	ich	388	5.2	0.208	3.4	NA	1.1	8.4	0.33	0.29	0.33	54.0
West: [Dundas Ro	d Link (W)										
10	L2	256	8.3	0.190	6.3	LOS A	0.9	6.8	0.30	0.58	0.30	52.0
12	R2	240	1.9	0.160	6.2	LOS A	0.4	3.2	0.32	0.68	0.32	48.6
Approa	ich	497	5.2	0.190	6.3	LOS A	0.9	6.8	0.31	0.63	0.31	50.7
All Veh	icles	1094	5.7	0.208	4.3	NA	1.1	8.4	0.26	0.41	0.26	52.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 79 of 128 Page 79 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2021_PM_noRavenSt]

Dundas Road/ Dundas Road Link_2021_PM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	lows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Dundas R	d (S)										
1	L2	236	0.0	0.245	5.5	LOS A	0.0	0.0	0.00	0.31	0.00	53.6
2	T1	220	4.1	0.245	0.0	LOS A	0.0	0.0	0.00	0.31	0.00	55.8
Approa	ch	456	2.0	0.245	2.9	NA	0.0	0.0	0.00	0.31	0.00	54.7
North:	Dundas Ro	d (N)										
8	T1	155	2.9	0.217	1.3	LOS A	1.2	9.2	0.52	0.39	0.52	52.9
9	R2	208	2.2	0.217	7.1	LOS A	1.2	9.2	0.52	0.39	0.52	53.4
Approa	ch	363	2.5	0.217	4.7	NA	1.2	9.2	0.52	0.39	0.52	53.2
West: [Dundas Ro	l Link (W)										
10	L2	482	5.8	0.371	6.7	LOS A	1.9	15.1	0.40	0.62	0.40	51.9
12	R2	53	2.1	0.037	6.3	LOS A	0.1	0.7	0.33	0.66	0.33	48.5
Approa	ich	535	5.5	0.371	6.7	LOS A	1.9	15.1	0.40	0.62	0.40	51.7
All Veh	icles	1354	3.5	0.371	4.9	NA	1.9	15.1	0.30	0.45	0.30	52.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 80 of 128 Page 80 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2021_AM]

Dundas Road/ Dundas Road Link_2021_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Dundas R	d (S)											
1	L2	49	0.0	0.119	5.5	LOS A	0.0	0.0	0.00	0.14	0.00	55.6	
2	T1	162	11.1	0.119	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	57.9	
Approa	ach	211	8.5	0.119	1.3	NA	0.0	0.0	0.00	0.14	0.00	57.3	
North:	Dundas R	d (N)											
8	T1	189	3.0	0.211	0.5	LOS A	1.1	8.7	0.34	0.30	0.34	54.2	
9	R2	204	7.1	0.211	6.3	LOS A	1.1	8.7	0.34	0.30	0.34	53.8	
Approa	ach	393	5.1	0.211	3.5	NA	1.1	8.7	0.34	0.30	0.34	54.0	
West: I	Dundas Ro	d Link (W)											
10	L2	265	8.5	0.197	6.3	LOS A	0.9	7.1	0.30	0.58	0.30	52.0	
12	R2	240	1.9	0.160	6.3	LOS A	0.4	3.2	0.33	0.68	0.33	48.6	
Approa	ach	506	5.3	0.197	6.3	LOS A	0.9	7.1	0.31	0.63	0.31	50.7	
All Veh	icles	1110	5.9	0.211	4.4	NA	1.1	8.7	0.26	0.42	0.26	52.8	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 81 of 128 Page 81 of 128

abla Site: 101 [Dundas Road North/ Dundas Road Link_2021_PM]

Dundas Road/ Dundas Road Link_2021_PM Site Category: (None) Giveway / Yield (Two-Way)

Mover	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	lows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Dundas R	d (S)											
1	L2	237	0.0	0.238	5.5	LOS A	0.0	0.0	0.00	0.32	0.00	53.5	
2	T1	206	4.4	0.238	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	55.7	
Approa	ich	443	2.0	0.238	3.0	NA	0.0	0.0	0.00	0.32	0.00	54.5	
North:	Dundas Ro	d (N)											
8	T1	170	2.0	0.222	1.3	LOS A	1.2	9.4	0.50	0.37	0.50	53.1	
9	R2	207	2.2	0.222	7.1	LOS A	1.2	9.4	0.50	0.37	0.50	53.5	
Approa	ich	376	2.1	0.222	4.5	NA	1.2	9.4	0.50	0.37	0.50	53.4	
West: [Dundas Ro	Link (W)											
10	L2	472	5.7	0.358	6.6	LOS A	1.9	14.5	0.39	0.61	0.39	52.0	
12	R2	48	2.3	0.034	6.3	LOS A	0.1	0.6	0.33	0.66	0.33	48.5	
Approa	ich	520	5.4	0.358	6.6	LOS A	1.9	14.5	0.38	0.62	0.38	51.7	
All Veh	icles	1339	3.4	0.358	4.8	NA	1.9	14.5	0.29	0.45	0.29	52.9	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Dundas Road North/ Dundas Road Link_2031_AM]

Dundas Road/ Dundas Road Link_2031_AM Site Category: (None) Giveway / Yield (Two-Way)

Mover	Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Dundas R	d (S)											
1	L2	60	1.9	0.171	5.6	LOS A	0.0	0.0	0.00	0.13	0.00	55.2	
2	T1	216	25.0	0.171	0.0	LOS A	0.0	0.0	0.00	0.13	0.00	57.8	
Approa	ich	275	20.0	0.171	1.2	NA	0.0	0.0	0.00	0.13	0.00	57.2	
North:	Dundas Ro	d (N)											
8	T1	274	4.9	0.308	0.9	LOS A	1.8	13.9	0.43	0.31	0.43	53.9	
9	R2	266	9.7	0.308	6.8	LOS A	1.8	13.9	0.43	0.31	0.43	53.4	
Approa	ich	540	7.3	0.308	3.8	NA	1.8	13.9	0.43	0.31	0.43	53.6	
West: [Dundas Ro	l Link (W)											
10	L2	461	9.8	0.370	6.9	LOS A	1.9	15.5	0.42	0.63	0.42	51.5	
12	R2	257	0.4	0.195	6.7	LOS A	0.5	3.9	0.40	0.74	0.40	48.4	
Approa	ich	718	6.4	0.370	6.8	LOS A	1.9	15.5	0.42	0.67	0.42	50.7	
All Veh	icles	1534	9.2	0.370	4.7	NA	1.9	15.5	0.35	0.45	0.35	52.6	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Dundas Road North/ Dundas Road Link_2031_PM]

Dundas Road/ Dundas Road Link_2031_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Dundas R	d (S)										
1	L2	242	0.0	0.327	5.5	LOS A	0.0	0.0	0.00	0.24	0.00	54.3
2	T1	349	9.6	0.327	0.0	LOS A	0.0	0.0	0.00	0.24	0.00	56.5
Approa	ich	591	5.7	0.327	2.3	NA	0.0	0.0	0.00	0.24	0.00	55.5
North:	Dundas R	d (N)										
8	T1	222	5.1	0.297	2.1	LOS A	1.8	14.2	0.60	0.40	0.63	52.8
9	R2	226	4.0	0.297	8.2	LOS A	1.8	14.2	0.60	0.40	0.63	53.2
Approa	ich	448	4.5	0.297	5.2	NA	1.8	14.2	0.60	0.40	0.63	53.0
West: [Dundas Ro	Link (W)										
10	L2	379	4.2	0.335	7.5	LOS A	1.7	12.8	0.49	0.71	0.52	51.8
12	R2	52	0.0	0.041	6.7	LOS A	0.1	0.8	0.39	0.71	0.39	48.5
Approa	ich	430	3.7	0.335	7.4	LOS A	1.7	12.8	0.48	0.71	0.50	51.5
All Veh	icles	1470	4.7	0.335	4.7	NA	1.8	14.2	0.32	0.43	0.34	53.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 84 of 128 Page 84 of 128

SITE LAYOUT

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_AM]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_AM Site Category: (None) Roundabout



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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 85 of 128 Page 85 of 128

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road 2021 AM noRavenSt]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_AM_noRavenSt Site Category: (None) Roundabout

Move	ment F	Performanc	e - Veh	icles								
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		ven/n	%	V/C	sec		ven	m				km/n
South	: Dunda:	s Road/ Con	structior	n Site Acce	ss (S)							
2	T1	101	0.0	0.156	0.7	LOS A	1.0	7.4	0.33	0.34	0.33	42.3
3	R2	112	0.0	0.156	5.7	LOS A	1.0	7.4	0.33	0.34	0.33	45.4
Appro	ach	213	0.0	0.156	3.3	LOS A	1.0	7.4	0.33	0.34	0.33	44.0
East:	Maida V	ale Rd (E)										
4	L2	773	0.0	0.772	9.9	LOS A	11.3	84.9	0.90	0.92	1.15	41.4
6	R2	109	15.5	0.772	16.3	LOS B	11.3	84.9	0.90	0.92	1.15	35.1
Appro	ach	882	1.9	0.772	10.7	LOS B	11.3	84.9	0.90	0.92	1.15	40.5
North:	Dundas	s Road (N)										
7	L2	47	21.4	0.303	4.3	LOS A	2.0	14.8	0.32	0.40	0.32	44.4
8	T1	384	0.0	0.303	4.1	LOS A	2.0	14.8	0.32	0.40	0.32	50.9
Appro	ach	431	2.3	0.303	4.1	LOS A	2.0	14.8	0.32	0.40	0.32	50.0
All Ve	hicles	1527	1.8	0.772	7.8	LOS A	11.3	84.9	0.66	0.69	0.80	43.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 86 of 128 Page 86 of 128

MOVEMENT SUMMARY

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road 2021 PM noRavenSt]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_PM_noRavenSt Site Category: (None) Roundabout

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
Couth	Dundaa	veh/h	%	V/C	sec		veh	m				km/r
South	. Dundas	Road/ Con	struction	Sile Acce	ss (5)							
2	T1	384	0.0	0.641	0.8	LOS A	6.8	49.6	0.39	0.38	0.39	41.4
3	R2	620	0.0	0.641	5.8	LOS A	6.8	49.6	0.39	0.38	0.39	44.5
Appro	ach	1004	0.0	0.641	3.9	LOS A	6.8	49.6	0.39	0.38	0.39	43.4
East:	Maida Val	le Rd (E)										
4	L2	112	0.0	0.134	3.8	LOS A	0.8	6.3	0.29	0.51	0.29	47.0
6	R2	70	12.9	0.134	9.8	LOS A	0.8	6.3	0.29	0.51	0.29	39.5
Appro	ach	182	4.9	0.134	6.1	LOS A	0.8	6.3	0.29	0.51	0.29	43.9
North:	Dundas	Road (N)										
7	L2	107	5.3	0.229	6.9	LOS A	1.5	11.2	0.70	0.70	0.70	44.4
8	T1	101	0.0	0.229	6.8	LOS A	1.5	11.2	0.70	0.70	0.70	46.8
Appro	ach	208	2.7	0.229	6.9	LOS A	1.5	11.2	0.70	0.70	0.70	45.4
All Ve	hicles	1394	1.0	0.641	4.6	LOS A	6.8	49.6	0.42	0.44	0.42	43.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 87 of 128 Page 87 of 128

MOVEMENT SUMMARY

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_AM]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_AM Site Category: (None) Roundabout

Move	Novement Performance - Vehicles														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
South:	Dundas F	Road/ Cons	struction	Site Acce	ss (S)										
2	T1	101	0.0	0.148	0.7	LOS A	1.0	7.0	0.33	0.33	0.33	42.5			
3	R2	101	0.0	0.148	5.7	LOS A	1.0	7.0	0.33	0.33	0.33	45.5			
Approa	ach	202	0.0	0.148	3.2	LOS A	1.0	7.0	0.33	0.33	0.33	44.1			
East: N	Maida Vale	Rd (E)													
4	L2	762	0.0	0.763	9.7	LOS A	10.9	81.7	0.88	0.91	1.12	41.7			
6	R2	110	16.3	0.763	16.1	LOS B	10.9	81.7	0.88	0.91	1.12	35.2			
Approa	ach	872	2.1	0.763	10.5	LOS B	10.9	81.7	0.88	0.91	1.12	40.7			
North:	Dundas R	oad (N)													
7	L2	45	22.5	0.297	4.2	LOS A	1.9	14.5	0.30	0.40	0.30	44.4			
8	T1	384	0.0	0.297	4.0	LOS A	1.9	14.5	0.30	0.40	0.30	51.2			
Approa	ach	429	2.4	0.297	4.0	LOS A	1.9	14.5	0.30	0.40	0.30	50.2			
All Veh	nicles	1503	1.9	0.763	7.7	LOS A	10.9	81.7	0.64	0.68	0.78	43.3			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_PM]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2021_PM Site Category: (None) Roundabout

Move	lovement Performance - Vehicles lov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
South:	: Dundas F	Road/ Cons	struction	Site Acce	ss (S)										
2	T1	384	0.0	0.630	0.7	LOS A	6.6	48.4	0.35	0.37	0.35	41.6			
3	R2	620	0.0	0.630	5.7	LOS A	6.6	48.4	0.35	0.37	0.35	44.7			
Approa	ach	1004	0.0	0.630	3.8	LOS A	6.6	48.4	0.35	0.37	0.35	43.6			
East: I	Maida Vale	e Rd (E)													
4	L2	101	0.0	0.119	3.8	LOS A	0.7	5.5	0.29	0.50	0.29	47.2			
6	R2	58	15.4	0.119	9.8	LOS A	0.7	5.5	0.29	0.50	0.29	39.4			
Approa	ach	160	5.6	0.119	6.0	LOS A	0.7	5.5	0.29	0.50	0.29	44.1			
North:	Dundas F	Road (N)													
7	L2	117	3.8	0.236	6.9	LOS A	1.5	11.5	0.70	0.70	0.70	44.7			
8	T1	101	0.0	0.236	6.9	LOS A	1.5	11.5	0.70	0.70	0.70	46.8			
Appro	ach	218	2.1	0.236	6.9	LOS A	1.5	11.5	0.70	0.70	0.70	45.6			
All Veh	nicles	1382	1.0	0.630	4.5	LOS A	6.6	48.4	0.40	0.43	0.40	43.9			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2031_AM]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2031_AM Site Category: (None) Roundabout

Move	lovement Performance - Vehicles /ov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
South:	Dundas F	Road/ Cons	struction	Site Acce	ss (S)										
2	T1	101	0.0	0.160	1.0	LOS A	1.0	7.7	0.42	0.37	0.42	41.9			
3	R2	101	0.0	0.160	6.0	LOS A	1.0	7.7	0.42	0.37	0.42	45.0			
Approa	ach	202	0.0	0.160	3.5	LOS A	1.0	7.7	0.42	0.37	0.42	43.5			
East: N	Maida Vale	e Rd (E)													
4	L2	762	0.0	0.826	11.8	LOS B	14.2	111.1	0.96	1.01	1.33	39.1			
6	R2	157	32.9	0.826	19.0	LOS B	14.2	111.1	0.96	1.01	1.33	32.6			
Approa	ach	919	5.6	0.826	13.0	LOS B	14.2	111.1	0.96	1.01	1.33	37.8			
North:	Dundas R	oad (N)													
7	L2	133	10.2	0.357	4.1	LOS A	2.5	19.2	0.33	0.41	0.33	46.5			
8	T1	384	0.0	0.357	4.0	LOS A	2.5	19.2	0.33	0.41	0.33	51.0			
Approa	ach	517	2.6	0.357	4.0	LOS A	2.5	19.2	0.33	0.41	0.33	49.6			
All Veh	nicles	1638	4.0	0.826	9.0	LOS A	14.2	111.1	0.70	0.74	0.90	41.3			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

Site: 101 [Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2031_PM]

Dundas Road/ Maida Vale Road/ Proposed Carpark Service Road_2031_PM Site Category: (None) Roundabout

Move	ment Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Dundas R	oad/ Cons	struction	Site Acce	ss (S)							
2	T1	384	0.0	0.752	2.9	LOS A	9.4	69.4	0.75	0.61	0.79	39.2
3	R2	620	0.0	0.752	7.9	LOS A	9.4	69.4	0.75	0.61	0.79	42.4
Approa	ach	1004	0.0	0.752	6.0	LOS A	9.4	69.4	0.75	0.61	0.79	41.3
East: N	Maida Vale	Rd (E)										
4	L2	101	0.0	0.225	3.9	LOS A	1.5	12.4	0.32	0.55	0.32	41.5
6	R2	200	16.3	0.225	9.9	LOS A	1.5	12.4	0.32	0.55	0.32	32.3
Approa	ach	301	10.8	0.225	7.9	LOS A	1.5	12.4	0.32	0.55	0.32	35.0
North:	Dundas R	oad (N)										
7	L2	166	6.8	0.320	7.1	LOS A	2.3	17.9	0.79	0.75	0.79	43.7
8	T1	101	0.0	0.320	7.0	LOS A	2.3	17.9	0.79	0.75	0.79	42.3
Approa	ach	267	4.2	0.320	7.1	LOS A	2.3	17.9	0.79	0.75	0.79	43.2
All Veh	nicles	1573	2.8	0.752	6.5	LOS A	9.4	69.4	0.67	0.62	0.70	40.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT

∇ Site: 101 [Maida Vale Road/ Ibis Place_2021_AM]

Maida Vale Road/ Ibis Place_2021_AM Site Category: (None) Giveway / Yield (Two-Way)



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abla Site: 101 [Maida Vale Road/ Ibis Place_2021_AM_noRavenSt]

Maida Vale Road/ Ibis Place_2021_AM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Mover	nent Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maida	a Vale Rd	(NE)									
25	T1	113	25.7	0.074	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	649	0.3	0.406	8.3	LOS A	3.0	22.1	0.59	0.75	0.67	42.9
Approa	ich	763	4.1	0.406	7.0	NA	3.0	22.1	0.50	0.64	0.57	45.0
NorthW	/est: Maid	a Vale Rd	(NW)									
27	L2	136	0.0	0.100	6.2	LOS A	0.4	3.0	0.30	0.58	0.30	43.9
29	R2	24	42.9	0.050	12.2	LOS B	0.2	1.7	0.69	0.88	0.69	32.8
Approa	ich	160	6.3	0.100	7.1	LOS A	0.4	3.0	0.36	0.62	0.36	41.9
SouthV	Vest: Ibis	PI (SW)										
30	L2	233	6.3	0.245	6.4	LOS A	0.0	0.0	0.00	0.34	0.00	51.4
31	T1	197	10.9	0.245	0.0	LOS A	0.0	0.0	0.00	0.34	0.00	57.4
Approa	ich	429	8.4	0.245	3.5	NA	0.0	0.0	0.00	0.34	0.00	53.7
All Veh	icles	1352	5.7	0.406	5.9	NA	3.0	22.1	0.33	0.54	0.36	46.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Maida Vale Road/ Ibis Place_2021_PM_noRavenSt]

Maida Vale Road/ Ibis Place_2021_PM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Mover	ment Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maida	a Vale Rd	(NE)									
25	T1	93	25.3	0.060	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	142	0.0	0.090	7.6	LOS A	0.5	3.3	0.50	0.66	0.50	43.4
Approa	ach	235	10.0	0.090	4.6	NA	0.5	3.3	0.30	0.40	0.30	50.1
NorthW	Vest: Maid	la Vale Rd	(NW)									
27	L2	521	0.0	0.475	8.6	LOS A	3.3	24.1	0.57	0.84	0.76	41.6
29	R2	216	2.1	0.205	7.6	LOS A	0.8	5.8	0.51	0.77	0.51	41.5
Approa	ach	737	0.6	0.475	8.3	LOS A	3.3	24.1	0.56	0.82	0.69	41.6
SouthV	Vest: Ibis	PI (SW)										
30	L2	40	22.2	0.246	6.6	LOS A	0.0	0.0	0.00	0.06	0.00	52.4
31	T1	411	3.8	0.246	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	68.0
Approa	ach	452	5.5	0.246	0.6	NA	0.0	0.0	0.00	0.06	0.00	65.8
All Veh	icles	1424	3.7	0.475	5.2	NA	3.3	24.1	0.34	0.51	0.41	47.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Maida Vale Road/ Ibis Place_2021_AM]

Maida Vale Road/ Ibis Place_2021_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	ment Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maida	a Vale Rd	(NE)									
25	T1	74	39.4	0.054	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	656	0.3	0.408	8.3	LOS A	3.0	22.3	0.59	0.75	0.67	42.9
Approa	ach	730	4.3	0.408	7.4	NA	3.0	22.3	0.53	0.67	0.60	44.3
NorthV	Vest: Maid	a Vale Rd	(NW)									
27	L2	149	0.0	0.111	6.3	LOS A	0.5	3.4	0.31	0.58	0.31	43.9
29	R2	19	52.9	0.043	12.7	LOS B	0.1	1.6	0.70	0.88	0.70	31.7
Approa	ach	169	6.0	0.111	7.0	LOS A	0.5	3.4	0.36	0.62	0.36	42.0
SouthV	Vest: Ibis	PI (SW)										
30	L2	216	7.3	0.243	6.4	LOS A	0.0	0.0	0.00	0.32	0.00	51.5
31	T1	208	10.3	0.243	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	58.1
Approa	ach	424	8.8	0.243	3.3	NA	0.0	0.0	0.00	0.32	0.00	54.2
All Veh	icles	1322	5.9	0.408	6.0	NA	3.0	22.3	0.34	0.55	0.38	46.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Maida Vale Road/ Ibis Place_2021_PM]

Maida Vale Road/ Ibis Place_2021_PM Site Category: (None) Giveway / Yield (Two-Way)

Move	ment Pei	formanc	e - Vehi	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maid	a Vale Rd	(NE)									
25	T1	42	51.4	0.033	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	129	0.9	0.082	7.6	LOS A	0.4	3.1	0.50	0.65	0.50	43.2
Approa	ach	171	13.2	0.082	5.7	NA	0.4	3.1	0.38	0.49	0.38	47.0
NorthV	Vest: Maic	la Vale Rd	(NW)									
27	L2	517	0.0	0.472	8.6	LOS A	3.2	23.8	0.57	0.84	0.76	41.6
29	R2	220	2.0	0.198	7.3	LOS A	0.7	5.6	0.49	0.75	0.49	41.8
Approa	ach	737	0.6	0.472	8.2	LOS A	3.2	23.8	0.55	0.81	0.68	41.7
South\	Nest: Ibis	PI (SW)										
30	L2	30	25.9	0.243	6.6	LOS A	0.0	0.0	0.00	0.04	0.00	51.5
31	T1	409	6.0	0.243	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	68.4
Approa	ach	439	7.4	0.243	0.5	NA	0.0	0.0	0.00	0.04	0.00	66.6
All Veh	nicles	1347	4.4	0.472	5.4	NA	3.2	23.8	0.35	0.52	0.42	47.5

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 96 of 128 Page 96 of 128

abla Site: 101 [Maida Vale Road/ Ibis Place_2031_AM]

Maida Vale Road/ Ibis Place_2031_AM Site Category: (None) Giveway / Yield (Two-Way)

Mover	nent Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maida	a Vale Rd	(NE)									
25	T1	106	16.0	0.063	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	675	4.8	0.455	9.0	LOS A	3.8	29.4	0.63	0.82	0.81	41.3
Approa	ich	781	6.3	0.455	7.8	NA	3.8	29.4	0.55	0.71	0.70	43.3
NorthW	/est: Maid	a Vale Rd	(NW)									
27	L2	147	6.1	0.115	6.5	LOS A	0.5	3.7	0.33	0.59	0.33	42.9
29	R2	81	5.6	0.129	10.2	LOS B	0.4	3.4	0.67	0.87	0.67	38.1
Approa	ich	228	5.9	0.129	7.8	LOS A	0.5	3.7	0.45	0.69	0.45	41.1
SouthV	Vest: Ibis	PI (SW)										
30	L2	243	7.4	0.267	6.4	LOS A	0.0	0.0	0.00	0.32	0.00	51.4
31	T1	224	10.1	0.267	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	57.9
Approa	ich	466	8.7	0.267	3.4	NA	0.0	0.0	0.00	0.32	0.00	54.0
All Veh	icles	1475	7.0	0.455	6.4	NA	3.8	29.4	0.36	0.58	0.44	45.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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FAL-AURECON-TM-RPT-00002_0

Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 97 of 128 Page 97 of 128

abla Site: 101 [Maida Vale Road/ Ibis Place_2031_PM]

Maida Vale Road/ Ibis Place_2031_PM Site Category: (None) Giveway / Yield (Two-Way)

Mover	nent Per	formanc	e - Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthE	ast: Maida	a Vale Rd	(NE)									
25	T1	35	41.9	0.025	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
26	R2	226	12.9	0.168	8.3	LOS A	0.9	7.3	0.57	0.72	0.57	40.3
Approa	ich	261	16.8	0.168	7.2	NA	0.9	7.3	0.49	0.62	0.49	42.3
NorthW	/est: Maid	a Vale Rd	(NW)									
27	L2	474	0.2	0.443	8.5	LOS A	2.8	20.9	0.57	0.84	0.74	41.6
29	R2	204	1.1	0.206	7.9	LOS A	0.8	5.7	0.53	0.80	0.53	41.3
Approa	ich	679	0.5	0.443	8.3	LOS A	2.8	20.9	0.56	0.83	0.68	41.5
SouthV	Vest: Ibis I	PI (SW)										
30	L2	76	5.9	0.271	6.4	LOS A	0.0	0.0	0.00	0.09	0.00	57.0
31	T1	435	2.1	0.271	0.0	LOS A	0.0	0.0	0.00	0.09	0.00	66.2
Approa	ich	511	2.6	0.271	1.0	NA	0.0	0.0	0.00	0.09	0.00	64.3
All Veh	icles	1451	4.2	0.443	5.5	NA	2.8	20.9	0.35	0.53	0.40	46.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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FAL-AURECON-TM-RPT-00002_0

Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 98 of 128 Page 98 of 128

SITE LAYOUT

abla Site: 101 [Milner Road/ Sultana Road West_2018_AM]

Milner Road/ Sultana Road West_2018_AM Site Category: (None) Giveway / Yield (Two-Way)





FAL-AURECON-TM-RPT-00002_0

Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 99 of 128 Page 99 of 128

abla Site: 101 [Milner Road/ Sultana Road West_2018_AM]

Milner Road/ Sultana Road West_2018_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South	East: Su	Itana Road \	West											
21	L2	44	0.0	0.060	6.6	LOS A	0.2	1.5	0.34	0.61	0.34	55.3		
22	T1	1	0.0	0.060	4.9	LOS A	0.2	1.5	0.34	0.61	0.34	52.8		
23	R2	30	0.0	0.060	6.2	LOS A	0.2	1.5	0.34	0.61	0.34	54.9		
Approa	ach	75	0.0	0.060	6.4	LOS A	0.2	1.5	0.34	0.61	0.34	55.1		
NorthE	East: Mil	ner Road												
24	L2	4	0.0	0.184	6.5	LOS A	0.0	0.5	0.01	0.03	0.01	62.3		
25	T1	267	30.7	0.184	0.0	LOS A	0.0	0.5	0.01	0.03	0.01	69.5		
26	R2	7	0.0	0.184	6.3	LOS A	0.0	0.5	0.01	0.03	0.01	61.6		
Approa	ach	279	29.4	0.184	0.3	NA	0.0	0.5	0.01	0.03	0.01	69.1		
North\	Vest: Su	Itana Road	West											
27	L2	13	0.0	0.018	5.7	LOS A	0.1	0.4	0.12	0.57	0.12	56.1		
28	T1	1	0.0	0.018	4.9	LOS A	0.1	0.4	0.12	0.57	0.12	53.5		
29	R2	11	0.0	0.018	6.2	LOS A	0.1	0.4	0.12	0.57	0.12	55.6		
Approa	ach	26	0.0	0.018	5.9	LOS A	0.1	0.4	0.12	0.57	0.12	55.8		
South	West: M	ilner Road												
30	L2	9	0.0	0.048	7.0	LOS A	0.1	1.1	0.23	0.20	0.23	59.6		
31	T1	60	1.9	0.048	0.3	LOS A	0.1	1.1	0.23	0.20	0.23	66.1		
32	R2	21	0.0	0.048	6.9	LOS A	0.1	1.1	0.23	0.20	0.23	58.9		
Appro	ach	90	1.3	0.048	2.5	NA	0.1	1.1	0.23	0.20	0.23	63.5		
All Veh	nicles	470	17.7	0.184	2.0	NA	0.2	1.5	0.11	0.18	0.11	64.5		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Milner Road/ Sultana Road West_2018_PM]

Milner Road/ Sultana Road West_2018_PM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
South	Foot: Su	veh/h	% Noot	V/C	sec		veh	m				km/h		
South	East. Su		vesi			1001								
21	L2	40	0.0	0.060	6.0	LOSA	0.2	1.5	0.25	0.59	0.25	55.6		
22	T1	1	0.0	0.060	5.0	LOS A	0.2	1.5	0.25	0.59	0.25	53.1		
23	R2	38	0.0	0.060	6.3	LOS A	0.2	1.5	0.25	0.59	0.25	55.2		
Appro	ach	80	0.0	0.060	6.2	LOS A	0.2	1.5	0.25	0.59	0.25	55.4		
NorthE	East: Mil	ner Road												
24	L2	19	0.0	0.108	6.6	LOS A	0.1	0.7	0.05	0.10	0.05	61.2		
25	T1	151	20.9	0.108	0.0	LOS A	0.1	0.7	0.05	0.10	0.05	68.1		
26	R2	10	0.0	0.108	6.7	LOS A	0.1	0.7	0.05	0.10	0.05	60.5		
Appro	ach	180	17.5	0.108	1.1	NA	0.1	0.7	0.05	0.10	0.05	66.8		
North	Vest: Su	Itana Road	West											
27	L2	11	0.0	0.022	6.1	LOS A	0.1	0.5	0.27	0.59	0.27	55.6		
28	T1	2	0.0	0.022	5.0	LOS A	0.1	0.5	0.27	0.59	0.27	53.1		
29	R2	15	0.0	0.022	6.3	LOS A	0.1	0.5	0.27	0.59	0.27	55.2		
Approa	ach	28	0.0	0.022	6.1	LOS A	0.1	0.5	0.27	0.59	0.27	55.2		
South	West: Mi	ilner Road												
30	L2	22	0.0	0.142	6.8	LOS A	0.4	3.0	0.16	0.18	0.16	60.1		
31	T1	192	2.9	0.142	0.2	LOS A	0.4	3.0	0.16	0.18	0.16	66.7		
32	R2	58	0.0	0.142	6.6	LOS A	0.4	3.0	0.16	0.18	0.16	59.4		
Appro	ach	273	2.1	0.142	2.1	NA	0.4	3.0	0.16	0.18	0.16	64.4		
All Veh	nicles	561	6.6	0.142	2.5	NA	0.4	3.0	0.15	0.23	0.15	63.1		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Milner Road/ Sultana Road West_2021_AM_noRavenSt]

Milner Road/ Sultana Road West_2021_AM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	East: Sul	tana Road V	Vest										
21	L2	8	0.0	0.299	6.6	LOS A	1.1	7.7	0.44	0.75	0.49	56.0	
22	T1	310	0.0	0.299	6.0	LOS A	1.1	7.7	0.44	0.75	0.49	53.4	
23	R2	7	0.0	0.299	7.4	LOS A	1.1	7.7	0.44	0.75	0.49	55.5	
Approa	ach	325	0.0	0.299	6.0	LOS A	1.1	7.7	0.44	0.75	0.49	53.5	
NorthE	East: Milr	ner Road											
24	L2	115	0.0	0.269	6.7	LOS A	1.3	9.4	0.24	0.31	0.24	58.3	
25	T1	231	3.4	0.269	0.3	LOS A	1.3	9.4	0.24	0.31	0.24	64.4	
26	R2	176	0.6	0.269	6.6	LOS A	1.3	9.4	0.24	0.31	0.24	57.6	
Approa	ach	522	1.7	0.269	3.8	NA	1.3	9.4	0.24	0.31	0.24	60.6	
NorthV	Vest: Su	ltana Road V	Vest										
27	L2	10	0.0	0.313	5.9	LOS A	1.1	8.9	0.40	0.76	0.47	54.9	
28	T1	118	0.0	0.313	6.1	LOS A	1.1	8.9	0.40	0.76	0.47	52.4	
29	R2	174	8.4	0.313	8.4	LOS A	1.1	8.9	0.40	0.76	0.47	52.0	
Approa	ach	302	4.8	0.313	7.4	LOS A	1.1	8.9	0.40	0.76	0.47	52.2	
South\	West: Mi	Iner Road											
30	L2	93	9.6	0.093	6.7	LOS A	0.2	1.4	0.16	0.38	0.16	56.9	
31	T1	49	6.8	0.093	0.3	LOS A	0.2	1.4	0.16	0.38	0.16	63.9	
32	R2	20	0.0	0.093	7.1	LOS A	0.2	1.4	0.16	0.38	0.16	57.2	
Approa	ach	163	7.6	0.093	4.8	NA	0.2	1.4	0.16	0.38	0.16	58.9	
All Veh	nicles	1312	2.7	0.313	5.3	NA	1.3	9.4	0.32	0.53	0.34	56.5	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Milner Road/ Sultana Road West_2021_PM_noRavenSt]

Milner Road/ Sultana Road West_2021_PM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	East: Sul	tana Road V	Vest										
21	L2	47	0.0	0.178	6.2	LOS A	0.6	4.2	0.37	0.68	0.37	55.6	
22	T1	118	0.0	0.178	6.3	LOS A	0.6	4.2	0.37	0.68	0.37	53.1	
23	R2	16	0.0	0.178	8.2	LOS A	0.6	4.2	0.37	0.68	0.37	55.2	
Approa	ach	181	0.0	0.178	6.4	LOS A	0.6	4.2	0.37	0.68	0.37	53.9	
NorthE	East: Milr	ner Road											
24	L2	122	0.0	0.271	7.3	LOS A	1.4	10.5	0.42	0.32	0.42	57.4	
25	T1	199	1.7	0.271	0.8	LOS A	1.4	10.5	0.42	0.32	0.42	63.4	
26	R2	175	0.0	0.271	7.4	LOS A	1.4	10.5	0.42	0.32	0.42	56.8	
Approa	ach	497	0.7	0.271	4.7	NA	1.4	10.5	0.42	0.32	0.42	59.4	
NorthV	Vest: Su	ltana Road V	Vest										
27	L2	16	0.0	0.513	7.3	LOS A	2.5	18.7	0.58	0.90	0.87	54.2	
28	T1	309	0.0	0.513	7.7	LOS A	2.5	18.7	0.58	0.90	0.87	51.7	
29	R2	158	4.3	0.513	9.3	LOS A	2.5	18.7	0.58	0.90	0.87	52.5	
Approa	ach	483	1.4	0.513	8.2	LOS A	2.5	18.7	0.58	0.90	0.87	52.1	
South\	Nest: Mi	ner Road											
30	L2	148	0.8	0.210	6.6	LOS A	0.3	2.5	0.12	0.26	0.12	59.2	
31	T1	211	3.2	0.210	0.2	LOS A	0.3	2.5	0.12	0.26	0.12	65.6	
32	R2	33	0.0	0.210	7.1	LOS A	0.3	2.5	0.12	0.26	0.12	58.6	
Approa	ach	392	2.0	0.210	3.2	NA	0.3	2.5	0.12	0.26	0.12	62.4	
All Veh	nicles	1553	1.2	0.513	5.6	NA	2.5	18.7	0.39	0.53	0.48	56.9	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 103 of 128 Page 103 of 128

abla Site: 101 [Milner Road/ Sultana Road West_2021_AM]

Milner Road/ Sultana Road West_2021_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South	East: Sul	tana Road \	Nest											
21	L2	8	0.0	0.327	7.2	LOS A	1.2	8.8	0.49	0.79	0.59	55.5		
22	T1	309	0.0	0.327	6.5	LOS A	1.2	8.8	0.49	0.79	0.59	52.9		
23	R2	7	0.0	0.327	8.0	LOS A	1.2	8.8	0.49	0.79	0.59	55.1		
Appro	ach	324	0.0	0.327	6.6	LOS A	1.2	8.8	0.49	0.79	0.59	53.0		
NorthE	East: Milr	er Road												
24	L2	124	0.0	0.333	6.7	LOS A	1.5	11.1	0.22	0.27	0.22	58.8		
25	T1	337	2.3	0.333	0.2	LOS A	1.5	11.1	0.22	0.27	0.22	65.1		
26	R2	189	0.6	0.333	6.6	LOS A	1.5	11.1	0.22	0.27	0.22	58.1		
Appro	ach	649	1.4	0.333	3.3	NA	1.5	11.1	0.22	0.27	0.22	61.7		
North	Vest: Sul	tana Road V	West											
27	L2	1	0.0	0.291	5.9	LOS A	1.0	7.8	0.53	0.82	0.61	54.5		
28	T1	117	0.0	0.291	6.5	LOS A	1.0	7.8	0.53	0.82	0.61	52.1		
29	R2	131	11.1	0.291	9.1	LOS A	1.0	7.8	0.53	0.82	0.61	50.9		
Appro	ach	249	5.9	0.291	7.9	LOS A	1.0	7.8	0.53	0.82	0.61	51.5		
South	West: Mil	ner Road												
30	L2	78	0.0	0.086	6.7	LOS A	0.2	1.5	0.20	0.34	0.20	57.9		
31	T1	56	8.0	0.086	0.5	LOS A	0.2	1.5	0.20	0.34	0.20	63.9		
32	R2	20	0.0	0.086	7.5	LOS A	0.2	1.5	0.20	0.34	0.20	57.2		
Appro	ach	154	2.9	0.086	4.5	NA	0.2	1.5	0.20	0.34	0.20	59.8		
All Veh	nicles	1376	2.0	0.333	5.0	NA	1.5	11.1	0.34	0.50	0.38	57.2		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Milner Road/ Sultana Road West_2021_PM]

Milner Road/ Sultana Road West_2021_PM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	East: Sult	ana Road V	/est										
21	L2	48	0.0	0.183	6.3	LOS A	0.6	4.3	0.39	0.69	0.39	55.5	
22	T1	117	0.0	0.183	6.4	LOS A	0.6	4.3	0.39	0.69	0.39	53.0	
23	R2	16	0.0	0.183	8.5	LOS A	0.6	4.3	0.39	0.69	0.39	55.1	
Approa	ach	181	0.0	0.183	6.6	LOS A	0.6	4.3	0.39	0.69	0.39	53.8	
NorthE	East: Miln	er Road											
24	L2	122	0.0	0.289	7.3	LOS A	1.5	11.2	0.41	0.31	0.41	57.6	
25	T1	227	1.5	0.289	0.8	LOS A	1.5	11.2	0.41	0.31	0.41	63.6	
26	R2	184	0.0	0.289	7.3	LOS A	1.5	11.2	0.41	0.31	0.41	57.0	
Approa	ach	534	0.6	0.289	4.5	NA	1.5	11.2	0.41	0.31	0.41	59.7	
North\	Vest: Sul	tana Road V	Vest										
27	L2	1	0.0	0.469	7.3	LOS A	2.1	15.4	0.60	0.91	0.87	54.3	
28	T1	309	0.0	0.469	7.7	LOS A	2.1	15.4	0.60	0.91	0.87	51.8	
29	R2	110	3.1	0.469	9.2	LOS A	2.1	15.4	0.60	0.91	0.87	52.9	
Approa	ach	420	0.8	0.469	8.1	LOS A	2.1	15.4	0.60	0.91	0.87	52.1	
South	West: Mil	ner Road											
30	L2	96	0.0	0.203	6.7	LOS A	0.3	2.6	0.13	0.20	0.13	59.9	
31	T1	249	3.2	0.203	0.2	LOS A	0.3	2.6	0.13	0.20	0.13	66.5	
32	R2	34	0.0	0.203	7.2	LOS A	0.3	2.6	0.13	0.20	0.13	59.3	
Appro	ach	379	2.1	0.203	2.4	NA	0.3	2.6	0.13	0.20	0.13	64.0	
All Vel	nicles	1513	1.0	0.469	5.2	NA	2.1	15.4	0.39	0.49	0.47	57.6	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT

Site: 101v [Milner Road/ Sultana Road West_2031_AM]

Milner Road/ Sultana Road West_2031_AM Site Category: (None) Signals - Fixed Time Isolated



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Site: 101v [Milner Road/ Sultana Road West_2031_AM]

Milner Road/ Sultana Road West_2031_AM

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 85 seconds (Site Practical Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed	
South	Fast: S	ultana Road \	70 Nest	V/C	Sec	_	ven	111	_	_	_	K111/11	
21	12	276	4.9	0 276	10 7	LOSB	42	32.8	0 47	0.69	0 47	51.6	
22	T1	371	4.5	0.856	41.6		13.9	107.5	0.98	0.93	1 19	35.7	
23	R2	8	0.0	0.856	49.6		13.9	107.5	1 00	1.00	1 27	35.5	
Appro	ach	655	4.6	0.856	28.6		13.0	107.5	0.76	0.83	0.80	41.0	
Арріо	acri	000	4.0	0.000	20.0	200.0	10.9	107.5	0.70	0.00	0.03	41.0	
North	East: Mi	ilner Road											
24	L2	117	0.0	0.821	37.2	LOS D	23.2	184.5	0.97	0.95	1.09	40.5	
25	T1	861	9.1	0.821	30.4	LOS C	23.2	184.5	0.92	0.90	1.07	43.9	
26	R2	376	1.5	0.901	50.0	LOS D	17.7	132.2	0.95	0.97	1.30	33.7	
Appro	ach	1354	6.2	0.901	36.4	LOS D	23.2	184.5	0.93	0.93	1.13	40.3	
North	West: S	ultana Road	West										
27	L2	1	0.0	0.798	53.7	LOS D	5.1	37.2	1.00	0.90	1.31	34.1	
28	T1	115	0.0	0.798	48.2	LOS D	5.1	37.2	1.00	0.90	1.31	33.5	
29	R2	97	2.3	0.798	54.1	LOS D	4.8	35.8	1.00	0.90	1.32	32.2	
Appro	ach	212	1.1	0.798	50.9	LOS D	5.1	37.2	1.00	0.90	1.32	32.9	
South	West: N	/lilner Road											
30	L2	208	1.1	0.900	56.2	LOS E	13.2	101.3	1.00	1.02	1.39	32.3	
31	T1	318	12.4	0.900	50.0	LOS D	13.2	101.3	1.00	1.03	1.40	35.5	
32	R2	124	4.5	0.848	56.3	LOS E	5.9	45.3	1.00	0.93	1.39	32.0	
Appro	ach	649	7.3	0.900	53.2	LOS D	13.2	106.5	1.00	1.00	1.39	33.7	
	hicles	2871	57	0 901	39.5		23.2	184 5	0.91	0 92	1 15	38.1	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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PHASING SUMMARY

Site: 101v [Milner Road/ Sultana Road West 2031 AM]

Milner Road/ Sultana Road West_2031_AM Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 85 seconds (Site Practical Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Opposed Turns - Copy Reference Phase: Phase A Input Phase Sequence: A, B, C, D, D1*, D2* Output Phase Sequence: A, B, C, D, D1* (* Variable Phase)

Phase Timing Summary

Phase	Α	В	С	D	D1
Phase Change Time (sec)	0	20	32	54	67
Green Time (sec)	14	6	16	7	12
Phase Time (sec)	20	12	22	13	18
Phase Split	24%	14%	26%	15%	21%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase

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Site: 101v [Milner Road/ Sultana Road West_2031_PM]

Milner Road/ Sultana Road West_2031_PM

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Practical Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	East: Si	ultana Road V	Vest										
21	L2	237	2.4	0.249	12.2	LOS B	4.1	30.8	0.50	0.70	0.50	51.1	
22	T1	118	1.9	0.784	49.6	LOS D	4.8	36.3	1.00	0.84	1.22	33.0	
23	R2	8	0.0	0.784	56.3	LOS E	4.8	36.3	1.00	0.88	1.29	33.3	
Approach		363	2.2	0.784	25.3	LOS C	4.8	36.3	0.67	0.75	0.75	43.0	
NorthE	East: Mi	Iner Road											
24	L2	126	0.0	0.900	53.3	LOS D	22.7	173.4	1.00	1.04	1.29	34.2	
25	T1	688	4.9	0.900	47.0	LOS D	22.7	173.4	1.00	1.04	1.30	36.6	
26	R2	182	0.6	0.444	39.6	LOS D	7.0	52.1	0.91	0.80	0.91	37.4	
Appro	ach	996	3.5	0.900	46.5	LOS D	22.7	173.4	0.98	1.00	1.23	36.4	
North	Vest: S	ultana Road V	Vest										
27	L2	147	0.0	0.899	56.0	LOS E	16.7	122.4	1.00	1.04	1.34	32.6	
28	T1	322	0.0	0.899	50.5	LOS D	16.7	122.4	1.00	1.04	1.34	32.1	
29	R2	176	0.0	0.899	56.0	LOS E	16.6	122.0	1.00	1.04	1.34	32.5	
Approa	ach	646	0.0	0.899	53.2	LOS D	16.7	122.4	1.00	1.04	1.34	32.3	
South	West: N	lilner Road											
30	L2	76	1.5	0.580	36.1	LOS D	10.3	79.3	0.92	0.81	1.08	40.7	
31	T1	482	4.9	0.580	31.1	LOS C	10.8	83.4	0.93	0.79	0.99	43.5	
32	R2	318	2.5	0.892	54.7	LOS D	16.0	120.8	1.00	0.97	1.33	32.5	
Appro	ach	876	3.7	0.892	40.1	LOS D	16.0	120.8	0.95	0.86	1.12	38.5	
All Veł	nicles	2881	2.6	0.900	43.4	LOS D	22.7	173.4	0.94	0.93	1.16	36.7	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park

PHASING SUMMARY

Site: 101v [Milner Road/ Sultana Road West_2031_PM]

Milner Road/ Sultana Road West_2031_PM Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Practical Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Opposed Turns - Copy - Copy Reference Phase: Phase A Input Phase Sequence: A, C1, C2, D, D1*, D2* Output Phase Sequence: A, C1, C2, D (* Variable Phase)

Phase Timing Summary

Phase	Α	C1	C2	D
Phase Change Time (sec)	0	29	41	64
Green Time (sec)	23	6	17	20
Phase Time (sec)	29	12	23	26
Phase Split	32%	13%	26%	29%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Output Phase Sequence Phase A REF Phase C2 Phase C1 Sultana Road West Milner Road Sultana Road West Milner Road Sultana Road West Milner Road >1~ 416 >14 410 >1~ 416 710 110 710 210 >10 >11 Milner Road Sultana Road West Milner Road Sultana Road West Milner Road Sultana Road West Phase D Sultana Road West Milner Road >1~ ×11

REF: Reference Phase

710

Milner Road

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>11

Sultana Road West

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SITE LAYOUT

igvee Site: 101 [Ibis Place/ Raven Street/ Maida Vale Road_2021_AM]

lbis Place/ Raven Street/ Maida Vale Road_2021_AM Site Category: (None) Giveway / Yield (Two-Way)



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abla Site: 101 [Ibis Place/ Raven Street/ Maida Vale Road_2021_AM]

Ibis Place/ Raven Street/ Maida Vale Road_2021_AM Site Category: (None) Giveway / Yield (Two-Way)

Mover	Movement Performance - Vehicles														
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
SouthE	ast: Rave	en St													
21	L2	70	1.6	0.076	7.7	LOS A	0.3	2.2	0.52	0.73	0.52	48.7			
23b	R3	17	0.0	0.076	7.5	LOS A	0.3	2.2	0.52	0.73	0.52	54.1			
Approa	ich	87	1.3	0.076	7.7	LOS A	0.3	2.2	0.52	0.73	0.52	50.0			
East: N	laida Vale	e Road													
4b	L3	48	0.0	0.379	7.4	LOS A	0.0	0.0	0.00	0.60	0.00	60.7			
4a	L1	635	5.0	0.379	5.6	LOS A	0.0	0.0	0.00	0.60	0.00	55.4			
Approa	ich	683	4.6	0.379	5.8	NA	0.0	0.0	0.00	0.60	0.00	55.9			
SouthV	Vest: Ibis	Place													
32a	R1	304	8.9	0.206	6.0	LOS A	0.4	3.3	0.19	0.48	0.19	52.4			
32	R2	37	3.0	0.206	9.2	LOS A	0.4	3.3	0.19	0.48	0.19	50.4			
Approa	ich	342	8.2	0.206	6.3	NA	0.4	3.3	0.19	0.48	0.19	52.2			
All Veh	icles	1111	5.5	0.379	6.1	NA	0.4	3.3	0.10	0.58	0.10	54.2			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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abla Site: 101 [Ibis Place/ Raven Street/ Maida Vale Road_2021_PM]

Ibis Place/ Raven Street/ Maida Vale Road_2021_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
SouthE	East: Rav	en St												
21	L2	4	0.0	0.053	5.9	LOS A	0.1	1.0	0.40	0.77	0.40	48.9		
23b	R3	53	0.0	0.053	7.8	LOS A	0.1	1.0	0.40	0.77	0.40	54.1		
Approa	ach	57	0.0	0.053	7.7	LOS A	0.1	1.0	0.40	0.77	0.40	53.8		
East: N	/laida Val	e Road												
4b	L3	45	0.0	0.128	7.4	LOS A	0.0	0.0	0.00	0.61	0.00	60.5		
4a	L1	167	14.1	0.128	5.7	LOS A	0.0	0.0	0.00	0.61	0.00	54.1		
Approa	ach	212	11.1	0.128	6.1	NA	0.0	0.0	0.00	0.61	0.00	55.8		
SouthV	Vest: Ibis	Place												
32a	R1	838	2.9	0.497	5.5	LOS A	0.9	6.7	0.12	0.51	0.12	55.2		
32	R2	89	1.3	0.497	7.2	LOS A	0.9	6.7	0.12	0.51	0.12	51.0		
Approa	ach	927	2.8	0.497	5.7	NA	0.9	6.7	0.12	0.51	0.12	54.8		
All Veh	icles	1197	4.1	0.497	5.8	NA	0.9	6.7	0.11	0.54	0.11	54.9		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 115 of 128 Page 115 of 128

abla Site: 101 [Ibis Place/ Raven Street/ Maida Vale Road_2031_AM]

Ibis Place/ Raven Street/ Maida Vale Road_2031_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
SouthE	East: Rave	en St													
21	L2	107	16.8	0.135	8.8	LOS A	0.5	4.4	0.58	0.79	0.58	46.7			
23b	R3	20	16.7	0.135	8.4	LOS A	0.5	4.4	0.58	0.79	0.58	48.9			
Approa	ach	127	16.8	0.135	8.7	LOS A	0.5	4.4	0.58	0.79	0.58	47.2			
East: N	/laida Vale	e Road													
4b	L3	46	19.5	0.403	7.7	LOS A	0.0	0.0	0.00	0.60	0.00	54.4			
4a	L1	673	4.7	0.403	5.6	LOS A	0.0	0.0	0.00	0.60	0.00	55.5			
Approa	ach	719	5.6	0.403	5.8	NA	0.0	0.0	0.00	0.60	0.00	55.4			
SouthV	Vest: Ibis	Place													
32a	R1	311	7.2	0.241	6.6	LOS A	0.9	6.7	0.32	0.44	0.32	52.0			
32	R2	64	12.3	0.241	10.2	LOS B	0.9	6.7	0.32	0.44	0.32	48.4			
Approa	ach	375	8.1	0.241	7.2	NA	0.9	6.7	0.32	0.44	0.32	51.3			
All Veh	icles	1221	7.5	0.403	6.5	NA	0.9	6.7	0.16	0.57	0.16	53.1			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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FAL-AURECON-TM-RPT-00002_0

Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 116 of 128 Page 116 of 128

abla Site: 101 [Ibis Place/ Raven Street/ Maida Vale Road_2031_PM]

Ibis Place/ Raven Street/ Maida Vale Road_2031_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
SouthE	East: Rave	en St												
21	L2	80	21.1	0.073	6.4	LOS A	0.3	2.4	0.26	0.60	0.26	47.9		
23b	R3	22	0.0	0.073	8.3	LOS A	0.3	2.4	0.26	0.60	0.26	55.0		
Approa	ach	102	16.5	0.073	6.8	LOS A	0.3	2.4	0.26	0.60	0.26	49.9		
East: N	/laida Vale	Road												
4b	L3	43	2.6	0.136	7.4	LOS A	0.0	0.0	0.00	0.61	0.00	59.6		
4a	L1	181	14.3	0.136	5.7	LOS A	0.0	0.0	0.00	0.61	0.00	54.1		
Approa	ach	224	12.1	0.136	6.1	NA	0.0	0.0	0.00	0.61	0.00	55.5		
SouthV	Vest: Ibis	Place												
32a	R1	854	1.6	0.544	5.8	LOS A	2.2	16.1	0.21	0.48	0.23	55.3		
32	R2	166	3.4	0.544	7.5	LOS A	2.2	16.1	0.21	0.48	0.23	50.3		
Approa	ach	1020	1.9	0.544	6.0	NA	2.2	16.1	0.21	0.48	0.23	54.4		
All Veh	icles	1346	4.7	0.544	6.1	NA	2.2	16.1	0.18	0.51	0.19	54.2		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 117 of 128 Page 117 of 128

SITE LAYOUT

abla Site: 101 [Dundas Road_Proposed Car Park Access 1_AM]

Dundas Road_Proposed Car Park Access 1_AM Site Category: (None) Giveway / Yield (Two-Way)



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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 118 of 128 Page 118 of 128

MOVEMENT SUMMARY

abla Site: 101 [Dundas Road_Proposed Car Park Access 1_AM]

Dundas Road_Proposed Car Park Access 1_AM Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles														
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
East: F	Proposed	Car Park													
4	L2	1	0.0	0.002	5.6	LOS A	0.0	0.1	0.69	0.52	0.69	25.7			
Approach		1	0.0	0.002	5.6	LOS A	0.0	0.1	0.69	0.52	0.69	25.7			
North:	Dundas F	Road													
7	L2	202	0.0	0.109	4.9	LOS A	0.0	0.0	0.00	0.55	0.00	34.8			
8	T1	944	0.0	0.484	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	39.9			
Approa	ach	1146	0.0	0.484	0.9	NA	0.0	0.0	0.00	0.10	0.00	39.0			
All Veh	nicles	1147	0.0	0.484	0.9	NA	0.0	0.1	0.00	0.10	0.00	39.0			

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

abla Site: 101 [Dundas Road_Proposed Car Park Access 1_PM]

Dundas Road_Proposed Car Park Access 1_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov	Turn	Demand F	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average	
U		lotal	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed	
		ven/n	%	V/C	sec		ven	m				Km/n	
East: F	roposea	Car Park											
4	L2	802	0.0	0.491	0.0	LOS A	3.4	25.2	0.02	0.00	0.02	30.2	
Approa	ach	802	0.0	0.491	0.0	LOS A	3.4	25.2	0.02	0.00	0.02	30.2	
North:	Dundas I	Road											
7	L2	202	0.0	0.081	4.9	LOS A	0.0	0.0	0.00	0.54	0.00	34.8	
8	T1	1	0.0	0.081	0.0	LOS A	0.0	0.0	0.00	0.54	0.00	33.6	
Approa	ach	203	0.0	0.081	4.8	NA	0.0	0.0	0.00	0.54	0.00	34.8	
All Veh	nicles	1006	0.0	0.491	1.0	NA	3.4	25.2	0.02	0.11	0.02	31.0	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 120 of 128 Page 120 of 128

SITE LAYOUT

∇ Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM Site Category: (None) Giveway / Yield (Two-Way)



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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 121 of 128 Page 121 of 128

MOVEMENT SUMMARY

V Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM _noRavenSt]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
NorthE	ast: Ibis F	Place											
25	T1	148	34.1	0.102	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0	
Approa	ch	148	34.1	0.102	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0	
NorthW	/est: Prop	osed Car I	Park										
27	L2	1	0.0	0.001	2.2	LOS A	0.0	0.0	0.50	0.28	0.50	28.7	
Approa	ich	1	0.0	0.001	2.2	LOS A	0.0	0.0	0.50	0.28	0.50	28.7	
SouthV	Vest: Ibis	Place											
31	T1	565	3.8	0.301	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9	
Approa	ich	565	3.8	0.301	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9	
All Veh	icles	715	10.1	0.301	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.7	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 122 of 128 Page 122 of 128

MOVEMENT SUMMARY

∇ Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_PM_noRavenSt]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_PM_noRavenSt Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
NorthE	ast: Ibis F	Place											
25	T1	309	9.1	0.173	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9	
Approa	ich	309	9.1	0.173	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9	
NorthW	/est: Prop	osed Car F	Park										
27	L2	142	0.0	0.135	1.9	LOS A	0.5	3.9	0.47	0.39	0.47	28.9	
Approa	ich	142	0.0	0.135	1.9	LOS A	0.5	3.9	0.47	0.39	0.47	28.9	
SouthV	Vest: Ibis	Place											
31	T1	449	3.5	0.239	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9	
Approa	ich	449	3.5	0.239	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9	
All Veh	icles	900	4.9	0.239	0.3	NA	0.5	3.9	0.07	0.06	0.07	56.3	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 123 of 128 Page 123 of 128

MOVEMENT SUMMARY

abla Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_AM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
NorthE	ast: Ibis	Place											
25	T1	104	48.4	0.080	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0	
Approa	ach	104	48.4	0.080	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0	
NorthW	Vest: Pro	posed Car	Park										
27	L2	1	0.0	0.001	2.2	LOS A	0.0	0.0	0.50	0.29	0.50	28.6	
Approa	ach	1	0.0	0.001	2.2	LOS A	0.0	0.0	0.50	0.29	0.50	28.6	
SouthV	Vest: Ibis	Place											
31	T1	576	3.7	0.307	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9	
Approa	ach	576	3.7	0.307	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9	
All Veh	icles	682	10.5	0.307	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.7	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Forrestfield Station Multi-Storey Car Park Forrestfield Station Multi-Storey Car Park Page 124 of 128 Page 124 of 128

MOVEMENT SUMMARY

∇ Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_PM]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2021_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
NorthE	ast: Ibis F	Place											
25	T1	262	9.9	0.148	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0	
Approa	ch	262	9.9	0.148	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0	
NorthW	/est: Prop	oosed Car F	Park										
27	L2	142	0.0	0.133	1.8	LOS A	0.5	3.9	0.47	0.38	0.47	28.9	
Approa	ch	142	0.0	0.133	1.8	LOS A	0.5	3.9	0.47	0.38	0.47	28.9	
SouthV	Vest: Ibis	Place											
31	T1	439	3.8	0.234	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9	
Approa	ch	439	3.8	0.234	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9	
All Veh	icles	843	5.1	0.234	0.3	NA	0.5	3.9	0.08	0.06	0.08	55.5	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

abla Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2031_AM]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2031_AM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthEast: Ibis Place												
25	T1	187	11.4	0.107	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
Approa	ich	187	11.4	0.107	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0
NorthWest: Proposed Car Park												
27	L2	1	0.0	0.001	2.8	LOS A	0.0	0.0	0.54	0.33	0.54	28.2
Approach		1	0.0	0.001	2.8	LOS A	0.0	0.0	0.54	0.33	0.54	28.2
SouthWest: Ibis Place												
31	T1	646	6.3	0.352	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9
Approa	ich	646	6.3	0.352	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9
All Veh	icles	834	7.4	0.352	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

∇ Site: 101 [Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2031_PM]

Proposed Car Park/ Ibis Place_Proposed Carpark Access 2_2031_PM Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand f Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
NorthEast: Ibis Place												
25	T1	239	7.0	0.131	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
Approa	ach	239	7.0	0.131	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0
NorthWest: Proposed Car Park												
27	L2	142	0.0	0.152	2.5	LOS A	0.6	4.3	0.53	0.48	0.53	28.4
Approa	ach	142	0.0	0.152	2.5	LOS A	0.6	4.3	0.53	0.48	0.53	28.4
SouthWest: Ibis Place												
31	T1	549	2.5	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9
Approa	ach	549	2.5	0.289	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9
All Veh	icles	930	3.3	0.289	0.4	NA	0.6	4.3	0.08	0.07	0.08	56.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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